

**PROPOSED HOUSING
DEVELOPMENT
SCHOLARSTOWN ROAD**

**DBFL
CONSULTING ENGINEERS**

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FOREWORD

The following Conditions and Notes on Site Investigation Procedures should be read in conjunction with this report.

General.

Recommendations made, and opinions expressed in the report are based on the strata observed in the exploratory holes, together with the results of in-situ and laboratory tests. No responsibility can be held for conditions which have not been revealed by exploratory work, or which occur between exploratory hole locations. Whilst the report may suggest the likely configuration of strata, both between exploratory hole locations, or below the maximum depth of the investigation, this is only indicative, and liability cannot be accepted for its accuracy.

Unless specifically stated, no account has been taken of possible subsidence due to mineral extraction below or close to the site.

Boring Procedures.

Unless otherwise stated, the 'Shell and Auger' technique of soft ground boring has been employed. All boring operations sampling and/or logging of soils and in-situ testing complies with the recommendations of the British Standard Code of Practice BS 5930 (1981), 'Site Investigation' and BS 1377:1990, 'Methods of test for soils for civil engineering purposes'.

Whilst the technique allows the maximum data to be obtained in soft ground, some disturbance and variation of soft and layered soils is unavoidable. Attention is drawn to this condition, whenever it is suspected. Where cobbles and boulders are recorded, no conclusion should be drawn concerning the size, presence, lithological nature, or numbers per unit volume of ground.

Where peat has been encountered during siteworks, samples have been logged in accordance with the Von Post Classification (ref. Von Post, L. 1992. Sveriges Geologiska Undersöknings torvinventering och några av dess hittills vunna resultat (SGU peat inventory and some preliminary results) Svenska Mosskulturforeningens Tidskrift, Jonkoping, Swedden, 36, 1-37 & Hobbs N. B. Mire morphology and the properties of some British and foreign peats. QJEG, Vol. 19, 1986).

Routine Sampling.

Undisturbed samples of soils, predominantly cohesive in nature are obtained unless otherwise stated by a 104mm diameter open-drive tube sampler. In granular soils, and where undisturbed sampling is inappropriate, disturbed samples are collected. Smaller disturbed samples are also recovered at intervals to allow a visual examination of the full strata section.

In-Situ Testing.

Standard penetration tests, utilising either the standard split spoon sampler or solid cone and automatic trip-hammer are conducted unless otherwise where required by instruction. Subsequent to a seating drive of 150mm, a summation for the number of blows for 300mm penetration is recorded on the boring records together with the blow count for each 75mm penetration. In cases where incomplete penetration is obtained, the number of blows for the recorded value of penetration are noted. In coarse granular soils, a cone end is fitted to the sampler and a similar procedure adopted.

Groundwater.

The depth of entry of any influx of groundwater is recorded during the course of boring operations. However, the normal rate of boring does not usually permit the recording of an equilibrium level for any one water strike. Where possible drilling is suspended for a period of twenty minutes to monitor the subsequent rise in water level.

Groundwater conditions observed in the borings or pits are those appertaining to the period of investigation. It should be noted however, that groundwater levels are subject to diurnal, seasonal and climatic variations and can also be affected by drainage condition, tidal variation or other causes.

Retention of Samples.

After satisfactory completion of all the scheduled laboratory tests on any sample, the remaining material is discarded unless a period of retention of samples is agreed, it is our normal practice to discard all soil samples one month after submission of our final report.

**REPORT ON A SITE INVESTIGATION
FOR A HOUSING DEVELOPMENT
AT SCHOLARSTOWN ROAD
RATHFARNHAM
DUBLIN**

DBFL CONSULTING ENGINEERS

Report No. 21167

OCTOBER 2017

I Introduction

A major residential development is planned for a greenfield site located at Scholarstown Road in Rathfarnham, Co. Dublin.

A comprehensive investigation of sub soil conditions in the area of development has been ordered by DBFL Consulting Engineers on behalf of their client.

The programme of the field investigation included the construction of boreholes, trial pits and dynamic probes to establish geotechnical criteria on which to base foundation and infra-structural design. Work was carried out in accordance with BS 5930, Code of Practice for Site Investigations (1999) and Eurocode 7.

In addition plate bearing tests were scheduled to determine in situ CBR values while soakaway testing was performed in accordance with BRE Digest 365.

A programme of laboratory testing to confirm geotechnical and environmental soil parameters followed site operations.

This report includes all factual data pertaining to the project and comments on the findings relative to foundation and infrastructural design.

II Fieldwork

The site is a green field one located off the Scholarstown Road, in the lands surrounding Beechpark House. The site fronts on to the main road to the south and is bounded by existing residential developments on the remaining sides. The location is noted on the Google Map in Appendix VII. Exploratory locations are shown on detailed drawings also contained in this Appendix. Details on these plan have been provided by DBFL.

Each location was electronically scanned to ensure that services were not disrupted. At borehole locations a trial pit was opened by hand to a depth of 1.20 metres to confirm this. All exploratory locations have been referenced to National Grid and OD levels established.

The scope of the field investigation included the following elements:

- * 3 nr. Conventional Boreholes
- * 15 nr. Machine Excavated Trial Pits
- * 15 nr. Plate Bearing Tests (In Situ CBR)
- 30 nr. H.D. Dynamic Probes
- * 6 nr. Percolation Tests to BRE Digest 365

Following the field operations samples were selected for laboratory analysis. This included standard geotechnical testing and detailed environmental analysis carried out by specialist laboratory.

a. Boreholes

The three exploratory holes were bored with conventional 200mm cable-tool methods using a Dando Exploratory Rig..

Detailed geotechnical records are contained in Appendix I to this report - the records give details of stratification, sampling, in-situ testing and groundwater. Note is also taken of any obstructions to normal boring requiring the use of the heavy chisel for advancement.

The records uniformly note surface topsoil overlying a thin stratum of firm brown sandy gravelly CLAY. Below approximately 1.00 metres the brown gravelly CLAY becomes stiff in consistency, becoming very stiff below about 3.00 metres. Boreholes were terminated at depths between 6.00 and 6.60 metres when further advancement was impractical.

The gravelly clay stratum is a glacial till, locally referred to as brown boulder clay and very typical of the area. The stratum contains random cobble and boulder material.

Ground water was not encountered during the course of the investigation. Long-term ground water observation was not required.

b. Trial Pits

Trial pits were opened at fifteen locations using a JCB excavator under geotechnical engineering supervision. Samples were recovered at intervals, ground water was noted where relevant and detailed trial pit records prepared. These records are contained in Appendix II to this report with accompanying photographs.

The trial pitting operation revealed a very consistent stratification pattern with topsoil overlying brown gravelly CLAY, initially firm in consistency but becoming stiffer below about 1.00 metres

Two trial pits however varied from the general pattern. TP03 and TP05 encountered FILL or made ground material extending from surface to 1.70 metres at TP03 and to 2.30 metres BGL at TP05. The FILL consists of brown boulder clay with brick and rubble including ceramic tiles.

In both locations firm to stiff brown gravelly CLAY was noted below the FILL and pits were terminated in this stratum.

All trial pits were dry and remained very stable. Some difficulty in excavation in the lower very stiff to hard boulder clay was experienced and trial pits were terminated generally between 2.50 and 2.80 metres.

c. Plate Bearing Tests

In situ CBR values were established by Plate Bearing Test at fifteen specified locations. Testing was carried out directly below the topsoil /organic zone at a depth of 0.40 metres.

A 450mm diameter steel plate is loaded incrementally and deflection is recorded. The plate is then off loaded and recovery recorded (Load Cycle). The process is then repeated (Re-Load Cycle).

The equivalent CBR value is then calculated for both cycles. Detailed results are presented in Appendix III and the data is summarised in the following Table A.

TABLE A

Test No.	CBR at Load Cycle (%)	CBR at RE-Load (%)
01	5.2	6.2
02	6.4	13.8
03	8.9	21.2
04	7.7	16.4
05	8.7	17.7
06	7.9	20.6
07	16.8	118.2
08	6.4	21.5
09	14.6	33.9
10	8.1	22.4
11	5.9	13.5
12	14.0	34.9
13	10.7	19.2
14	3.5	7.2
15	18.1	41.3
AVERAGE	8.6	20.7

The average value on reload excludes the very high result from PBT 07 which may be due to a boulder obstruction at the plate test location.

d. Dynamic Probes

Probing was carried out at thirty locations to establish a pattern of soil strength with depth. Probing was in accordance with the heavy-duty probe specification of BS 1377: Part 9: 1990. In these tests, the soil resistance is measured in terms of the number of drop-hammer blows required to drive the test probe through each 100 mm increment of penetration. Probing is terminated when the blow count exceeds 25/100mm to avoid damage to the apparatus. Where loose material is present a single blow count may drive the apparatus in excess of 100mm. In this instance blow counts of zero may be recorded.

Results are presented both numerically and graphically in Appendix IV.

The probes generally encountered stiff soils at relatively shallow depth. A Dynamic Probe resistance of $N_{100} = 5$, with no dramatic reduction in underlying strength is indicative of soils suitable for foundation for traditional two storey housing.

Some anomalies to the general pattern were noted as follows:

Probe No.	Soft Soils (N ₁₀₀ < 3)	Stiff Soils (N ₁₀₀ > 5)
DP06	0.70 – 2.20	2.20 – 3.20
DP08	0.80 – 1.30	1.40 – 2.30
DP20	0.80 – 1.00	1.00 – 1.60
DP23	1.00 – 2.10	2.20 – 3.10
DP29	0.90 – 1.20	1.30 – 2.70

e. BRE Digest 365 Soakaway

A total of six percolation tests were carried out as noted on the site plans.

Infiltration testing was performed in accordance with BRE Digest 365 'Soakaway Design'. To obtain a measure of the infiltration rate of the sub-soils, water is poured into the test pit, and records taken of the fall in water level against time. This operation is generally performed over two cycles of soakage and dispersion following initial soakage.

The infiltration rate is the volume of water dispersed per unit exposed area per unit of time, and is generally expressed as metres/minute or metres/second. In these calculations the exposed area is the sum of the base area and the average internal area of the pit sides over the test duration.

Records for each test are presented in Appendix V. The stratification and water table in each test pit is noted and the records of water level with time are recorded.

Designs are based on the slowest infiltration rate, which is generally calculated from the final cycle. The infiltration rate (f) is calculated and the results for the individual tests are noted on the record sheets.

Results confirm that little or no percolation is available in the cohesive boulder clays encountered on this site. The percolation characteristics are typical of the regional boulder clay deposition

III Testing

(a) In-Situ :

Standard penetration tests were carried out at approximate 1.00 metre intervals in the geotechnical boreholes to measure relative in-situ soil strength. N values are noted in the right hand column of the boring records, representing the blow count required to drive the standard sampler 300mm into the soil, following initial seating blows. Where full test penetration was not achieved the blow count for a specific penetration is recorded, or refusal is indicated where appropriate

The results of the tests are summarised as follows:

STRATUM	N VALUE RANGE	COMMENT
Brown gravelly CLAY (Boulder Clay)		
1.00m BGL	19 to 40	Stiff to Very Stiff
2.00m BGL	21 to 26	Stiff
3.00m BGL	25 to 34	Stiff
4.00m BGL	27 to 34	Stiff
5.00m BGL	29 to 56	Very Stiff to Hard
6.00m BGL	36 to 57	Very Stiff to Hard

(b) Geotechnical Laboratory :

All samples from the boreholes and trial pits have been returned to the IGSL laboratory for initial visual inspection, a schedule of testing was prepared and tests as appropriate carried out.

Geotechnical testing was carried out by IGSL in it's INAB accredited facility. Chemical and environmental testing was carried out by CHEMTEST Limited in their UKAS laboratory. The programme of testing included the following elements and all results are presented in Appendix VI.

- a. Moisture Content and Classification (Liquid and Plastic Limits)
- b. Particle size distribution (Sieve Analysis / Hydrometer)
- c. Sulphate and pH determination.
- d. RILTA Environmental Suite

Classification / M.C.

The liquid and plastic limits were established for several samples of the glacial till or boulder clay. Results generally fall into the CL Zone of the standard classification indicative of low plasticity clay matrix material. In two samples a silt matrix has been established. These minor variations are very typical of the glacial till stratum.

Moisture contents are in the range 8 to 14 %, again very normal for these glacial soils.

Grading

PSD curves were established by wet sieve and hydrometer analysis. The grading for the boulder clay is typically straight line, reflecting the heterogeneous nature of the stratum, with particle distribution from the clay to the gravel fraction. One sample from TP03 grades as a clay bound sandy GRAVEL, this variation is again typical of the regional glacial till.

Sulphate and pH.

Three samples were selected for sulphate and pH analysis. Sulphate concentrations (SO₄ 2:1 extract) of < 0.010 g/l were established with an average pH of 8.45. No special precautions are necessary to protect foundation concrete from sulphate aggression. A sulphate design class of DS-1 (ACEC Classification for Concrete) is indicated for concentrations less than 0.5 g/l.

Environmental

Five samples were submitted for detailed environmental analysis to RILTA (WAC) parameters. The results indicate that the material is essentially INERT with little or no elevated contaminant levels recorded. Material excavated from this site can be readily disposed of to a licensed landfill facility or used within the site and no problems are anticipated with safety of personnel operating on the site.

No asbestos was noted during routine asbestos screening.

IV Discussion

The proposed development is for traditional housing on a site located on Scholarstown Road in Rathfarnham.

The area is a greenfield one, bounded on three sides by existing housing developments.

A comprehensive investigation of ground conditions has been carried out on the instructions of DBFL involving Borehole and Trial Pit investigation with supporting Dynamic Probes, Plate Bearing Tests and Percolation Tests.

Summary Stratification

The investigation has confirmed that firm to stiff brown boulder CLAY is present at shallow depth over most of the site area. MADE ground was noted up to a depth of 2.20 metres at TP03 and TP04. Isolated variable soft zones were also identified by Dynamic Probing at DP06, DP08, DP20, DP23 and DP29. At DP06 and DP03 the soft areas extended to depths of about 2.00 metres.

Foundations:

Soil strength has been determined by SPT tests in the three boreholes and by Dynamic Probe resistance in the thirty locations examined. The visual assessment of the soils during trial pitting and the laboratory data is also utilised in assessing allowable bearing pressures.

Based on the data obtained we would suggest the use of traditional reinforced strip or pad foundations, founded at 0.80 to 1.00 metres BGL on the boulder clay. SPT values and Dynamic Probe data indicate an allowable bearing pressure of 150 kPa for the brown boulder clay at this depth. Total settlement of the order of 10mm can be expected under this load, differential movement should be negligible.

While the majority of probes and boreholes fall into the above category some variations were noted, with some MADE GROUND and or soft zones identified. The maximum depth of poor ground should be of the order of 2.00 metres.

Careful visual inspection of excavated formation is advised to ensure uniformity and suitability of the founding medium for each housing unit or units. All unsuitable material identified should be removed and replaced with low-grade concrete.

Ground Floor Slabs

The sub soils below the top-soil at a depth of about 0.50 metres should readily support lightly loaded ground floor slabs. All organic soils should be removed and imported hardcore infill should be suitably compacted and fully comply with current building regulations.

Excavation / Ground Water

The boulder clay deposits will remain stable during excavation for foundations or services. No ground water was encountered during the investigation and water ingress during foundation construction is not expected.

Statutory safety regulations, should however be noted, these prohibit personnel entering unsupported excavations greater than 1.20 metre deep, irrespective of apparent stability.

Roads and Pavements

CBR tests at shallow depth suggest that an average CBR of about 8% can be adopted for road and pavement construction. Again visual inspection to confirm suitability of formation is advised.

Percolation

BRE Digest 365 tests in six areas confirm that the sub soils will not be suitable for dispersion of storm and surface water in conventional soakaways. Disposal of storm or surface water to the Local Authority Drainage System is recommended.

Concrete

Tests indicate low sulphate concentrations and near neutral pH. No special precautions are deemed necessary to protect foundation concrete.

Environmental


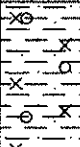


Tests carried out on samples from this site indicate that the soils are classified as INERT with extremely low contamination levels. Material excavated from this site can be disposed of to licensed landfill or utilised within the site for non-engineering purposes, landscaping etc.

IGSL/JC
October 2018

Appendix I Boring Records

 IGSL	GEOTECHNICAL BORING RECORD	REPORT NUMBER 21167
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CONTRACT Beech House, Scholarstown		BOREHOLE NO. BH01
		SHEET Sheet 1 of 1
CO-ORDINATES	RIG TYPE Dando	DATE COMMENCED 09/08/2018
GROUND LEVEL (m AOD)	BOREHOLE DIAMETER (mm) 200	DATE COMPLETED 09/08/2018
	BOREHOLE DEPTH (m) 6.40	
CLIENT	SPT HAMMER REF. NO.	BORED BY JO'T/MB
ENGINEER DBFL Consulting Eng.	ENERGY RATIO (%)	PROCESSED BY EK

Depth (m)	Description	Legend	Elevation	Depth (m)	Samples				Field Test Results	Standpipe Details
					Ref. Number	Sample Type	Depth (m)	Recovery		
0	TOPSOIL			0.20						
1	Firm brown SILT/CLAY with occasional fine gravel			1.50	AA93296	D	1.00	N = 23 (2, 3, 5, 6, 6, 6)		
2	Stiff brown sandy gravelly CLAY			2.00	AA93297	D	2.00	N = 26 (3, 4, 6, 7, 6, 7)		
3	Stiff to very stiff dark brownish grey laminated CLAY with occasional fine gravel				AA93298	D	3.00	N = 34 (4, 6, 7, 9, 9, 9)		
4					AA93299	D	4.00	N = 38 (3, 7, 8, 9, 10, 11)		
5					AA93300	D	5.00	N = 55 (4, 11, 10, 12, 16, 17)		
6				6.40	AA98251	D	6.00	N = 57 (2, 7, 12, 14, 16, 15)		
	End of Borehole at 6.40 m									

HARD STRATA BORING/CHISELLING				WATER STRIKE DETAILS					
From (m)	To (m)	Time (h)	Comments	Water Strike	Casing Depth	Sealed At	Rise To	Time (min)	Comments
6.2	6.4	1.5							No water strike

INSTALLATION DETAILS					GROUNDWATER PROGRESS				
Date	Tip Depth	RZ Top	RZ Base	Type	Date	Hole Depth	Casing Depth	Depth to Water	Comments

REMARKS CAT scan and inspection pit completed .	Sample Legend D - Small Disturbed (tub) B - Bulk Disturbed LB - Large Bulk Disturbed Env - Environmental Sample (Jar + Vial + Tub) UT - Undisturbed 100mm Diameter Sample P - Undisturbed Piston Sample W - Water Sample
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IGSL BH LOG 21167.GPJ IGSL.GDT 22/8/18



GEOTECHNICAL BORING RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown		BOREHOLE NO. BH02
CO-ORDINATES		SHEET Sheet 1 of 1
GROUND LEVEL (m AOD)	RIG TYPE Dando	DATE COMMENCED 10/08/2018
	BOREHOLE DIAMETER (mm) 200	DATE COMPLETED 13/08/2018
CLIENT	SPT HAMMER REF. NO.	BORED BY JO'T/MB
ENGINEER DBFL Consulting Eng.	ENERGY RATIO (%)	PROCESSED BY EK

Depth (m)	Description	Legend	Elevation	Depth (m)	Samples				Field Test Results	Standpipe Details
					Ref. Number	Sample Type	Depth (m)	Recovery		
0	TOPSOIL			0.20						
1	Firm brown SILT/CLAY with some gravel and occasional cobbles			1.20	AA98252	D	1.00		N = 19 (2, 3, 4, 5, 5, 5)	
2	Stiff brown gravelly cobbly CLAY				AA98253	D	2.00		N = 21 (2, 3, 5, 5, 5, 6)	
3					AA98254	D	3.00		N = 25 (3, 4, 5, 6, 7, 7)	
4	Very stiff brown gravelly laminated SILT/CLAY with occasional cobbles			3.50	AA98255	D	4.00		N = 34 (2, 6, 7, 7, 9, 11)	
5					AA98256	D	5.00		N = 45 (2, 4, 9, 11, 11, 14)	
6					AA98257	D	6.00		N = 46 (3, 7, 13, 9, 11, 13)	
6.60	End of Borehole at 6.60 m									

HARD STRATA BORING/CHISELLING				WATER STRIKE DETAILS					
From (m)	To (m)	Time (h)	Comments	Water Strike	Casing Depth	Sealed At	Rise To	Time (min)	Comments
4.4	4.5	0.75							No water strike
6.4	6.6	1.5							

INSTALLATION DETAILS					GROUNDWATER PROGRESS				
Date	Tip Depth	RZ Top	RZ Base	Type	Date	Hole Depth	Casing Depth	Depth to Water	Comments







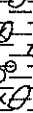
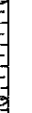
REMARKS CAT scan and inspection pit completed .

Sample Legend
 D - Small Disturbed (tub)
 B - Bulk Disturbed
 LB - Large Bulk Disturbed
 Env - Environmental Sample (Jar + Vial + Tub)
 UT - Undisturbed 100mm Diameter Sample
 P - Undisturbed Piston Sample
 W - Water Sample

IGSL BH LOG 21167.GPJ IGSL.GDT 22/8/18

 IGSL	GEOTECHNICAL BORING RECORD	REPORT NUMBER 21167
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CONTRACT Beech House, Scholarstown		BOREHOLE NO. BH03
		SHEET Sheet 1 of 1
CO-ORDINATES	RIG TYPE Dando	DATE COMMENCED 13/08/2018
GROUND LEVEL (m AOD)	BOREHOLE DIAMETER (mm) 200	DATE COMPLETED 14/08/2018
	BOREHOLE DEPTH (m) 6.00	
CLIENT	SPT HAMMER REF. NO.	BORED BY JO'T/MB
ENGINEER DBFL Consulting Eng.	ENERGY RATIO (%)	PROCESSED BY EK

Depth (m)	Description	Legend	Elevation	Depth (m)	Samples				Field Test Results	Standpipe Details
					Ref. Number	Sample Type	Depth (m)	Recovery		
0	TOPSOIL			0.20						
	Firm brown SILT/CLAY with occasional gravel			0.90						
1	Layer of gravelly CLAY with frequent cobbles			1.60	AA98258	D	1.00	N = 40 (2, 3, 4, 15, 15, 6)		
2	Stiff to very stiff brown laminated SILT/CLAY with fine to medium gravel and occasional cobbles				AA98259	D	2.00	N = 24 (3, 2, 4, 6, 7, 7)		
3					AA98260	D	3.00	N = 27 (2, 4, 5, 7, 8, 7)		
4					AA98261	D	4.00	N = 27 (2, 4, 6, 7, 7, 7)		
5					AA98262	D	5.00	N = 29 (2, 4, 7, 7, 8, 7)		
6	End of Borehole at 6.00 m			6.00	AA98263	D	6.00	N = 35 (3, 6, 7, 9, 9, 10)		

HARD STRATA BORING/CHISELLING				WATER STRIKE DETAILS					
From (m)	To (m)	Time (h)	Comments	Water Strike	Casing Depth	Sealed At	Rise To	Time (min)	Comments
1.1	1.6	1							No water strike

INSTALLATION DETAILS					GROUNDWATER PROGRESS				
Date	Tip Depth	RZ Top	RZ Base	Type	Date	Hole Depth	Casing Depth	Depth to Water	Comments

REMARKS CAT scan and inspection pit completed .	Sample Legend O - Small Disturbed (tub) B - Bulk Disturbed LB - Large Bulk Disturbed Env - Environmental Sample (Jar + Vat + Tub)	UT - Undisturbed 100mm Diameter Sample P - Undisturbed Piston Sample W - Water Sample
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Appendix II Trial Pit Records



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown		TRIAL PIT NO. TP01	
LOGGED BY EK		SHEET Sheet 1 of 1	
CLIENT ENGINEER DBFL Consulting Engineers		DATE STARTED 08/08/2018	
CO-ORDINATES 712,578.73 E 726,784.07 N		DATE COMPLETED 08/08/2018	
GROUND LEVEL (m) 83.45		EXCAVATION METHOD	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
	Firm to stiff brown gravelly CLAY with a low cobble content and a low boulder content. Cobbles are sub-angular and boulders are sub-angular to sub-rounded. Boulders are less than 400mm in size. (Possible made ground).		0.30	83.15						
1.0	Stiff brown gravelly CLAY with some cobbles and occasional boulders		1.20	82.25		AA94906	B	1.00		
2.0	Stiff purple mottled blue SILT/CLAY with gravel		2.50	80.95		AA94907	B	2.00		
3.0	End of Trial Pit at 2.80m		2.80	80.65		AA94908	B	2.80		

Groundwater Conditions

Stability
Good

General Remarks

Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL.GDT 15/8/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown		TRIAL PIT NO. TP02	
LOGGED BY K. Kinsella		SHEET Sheet 1 of 1	
CO-ORDINATES 712,550.65 E 726,800.61 N		DATE STARTED 10/08/2018	
GROUND LEVEL (m) 84.00		DATE COMPLETED 10/08/2018	
CLIENT ENGINEER DBFL Consulting Engineers		EXCAVATION METHOD	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	Firm brown TOPSOIL with rootlets									
	Stiff brown/light brown sandy SILT/CLAY with rare subangular to subrounded gravel		0.20	83.80		AA98917	B	0.40-0.50		
	Stiff brown sandy gravelly slightly cobbly SILT/CLAY with rare subrounded boulders up to 350mm, cobble content increases with depth		0.80	83.20		AA98918	B	1.00-1.10		
	End of Trial Pit at 2.70m		2.70	81.30		AA98919	B	2.50-2.60		

Groundwater Conditions
Dry

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL_GDT 15/8/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown		TRIAL PIT NO. TP03	
LOGGED BY EK		SHEET Sheet 1 of 1	
CLIENT ENGINEER DBFL Consulting Engineers		DATE STARTED 08/08/2018	
CO-ORDINATES 712,582.41 E 726,843.35 N		DATE COMPLETED 08/08/2018	
GROUND LEVEL (m) 82.45		EXCAVATION METHOD	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
0.20	MADE GROUND (Comprised of brown gravelly clay with a low cobble content. Contains brick fragments and tiles. Cobbles are less than 300mm and sub-angular to sub-rounded)		0.20	82.25						
1.0						AA94904	B	1.00		
1.70	Stiff brown gravelly CLAY with a high cobble content and a low boulder content. Cobbles are angular to sub-angular. Boulders are sub-angular and less than 500mm in size. (Possibly made ground).		1.70	80.75						
2.0						AA94905	B	2.00		
2.50	End of Trial Pit at 2.50m		2.50	79.95						
3.0										
4.0										

Groundwater Conditions

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL_TP_LOG_21167.GPJ_IGSL_GDT_15/08/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown		TRIAL PIT NO. TP04	
LOGGED BY K. Kinsella		SHEET Sheet 1 of 1	
CLIENT ENGINEER DBFL Consulting Engineers		CO-ORDINATES 712,580.24 E 726,937.92 N	DATE STARTED 10/08/2018
		GROUND LEVEL (m) 80.24	DATE COMPLETED 10/08/2018
EXCAVATION METHOD			

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	Firm brown TOPSOIL with rootlets									
	Stiff brown/light brown sandy very gravelly SILT with subangular to subrounded cobbles up to 120mm		0.20	80.04						
	Stiff to locally firm brown sandy gravelly CLAY with cobbles and rare subrounded boulders up to 450mm		0.45	79.79		AA98920	B	0.30-0.40		
1.0										
						AA98921	B	1.20-1.30		
2.0										
	End of Trial Pit at 2.20m		2.20	78.04		AA98922	B	2.10-2.20		
3.0										
4.0										

Groundwater Conditions
Dry

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL GDT 15/08/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown		TRIAL PIT NO. TP05	
LOGGED BY EK		SHEET Sheet 1 of 1	
CO-ORDINATES 712,536.88 E 726,860.16 N		DATE STARTED 08/08/2018	
GROUND LEVEL (m) 83.09		DATE COMPLETED 08/08/2018	
CLIENT ENGINEER DBFL Consulting Engineers		EXCAVATION METHOD	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
0.30	Firm to stiff MADE GROUND in a gravelly clay matrix with a low cobble content. Contains brick fragments and tiles. Cobbles are less than 200mm and rounded to sub-rounded.		0.30	82.79						
1.0						AA94901	B	1.00		
2.0						AA94902	B	2.00		
2.30	Firm to stiff greyish brown gravelly CLAY with a low cobble content. Cobbles are angular to sub-angular and less than 200mm in size. (Possibly made ground).		2.30	80.79						
2.80	End of Trial Pit at 2.80m		2.80	80.29		AA94903	B	2.70		
3.0										
4.0										


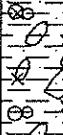


Groundwater Conditions

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL_TP_LOG_21167.GPJ_IGSL_GDT_15/08/18

CONTRACT Beech House, Scholarstown	CO-ORDINATES 712,333.26 E 726,830.99 N	TRIAL PIT NO. TP06 SHEET Sheet 1 of 1	DATE STARTED 07/08/2018 DATE COMPLETED 07/08/2018
LOGGED BY EK	GROUND LEVEL (m) 86.08	EXCAVATION METHOD	
CLIENT ENGINEER DBFL Consulting Engineers			

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
	Firm to stiff brown slightly gravelly SILT/CLAY		0.30	85.78						
1.0	Firm to stiff greyish brown slightly gravelly CLAY with a low cobble content and rare shells. Cobbles are sub-angular to sub-rounded and less than 300mm.		1.00	85.08		AA81273	B	1.00		
2.0						AA81274	B	2.00		
	Firm to stiff grey slightly gravelly CLAY with a low cobble content. Cobbles are sun-angular to sub-rounded and less than 300mm.		2.40 2.50	83.68 83.58		AA81275	B	2.50		
	End of Trial Pit at 2.50m									

Groundwater Conditions

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL GDT 15/8/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown		TRIAL PIT NO. TP07	
LOGGED BY EK		SHEET Sheet 1 of 1	
CLIENT ENGINEER DBFL Consulting Engineers		DATE STARTED 07/08/2018	
CO-ORDINATES 712,385.49 E 726,811.90 N		DATE COMPLETED 07/08/2018	
GROUND LEVEL (m) 85.82		EXCAVATION METHOD	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (kPa)	Hand Penetrometer (kPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
0.30	Firm to stiff greyish brown gravelly CLAY with a medium cobble content. Cobbles are angular to sub-angular and less than 300mm in size.		0.30	85.52						
1.0						AA81276	B	1.00		
2.0						AA81277	B	2.00		
2.50	End of Trial Pit at 2.50m		2.50	83.32						

Groundwater Conditions

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL.GDT 15/8/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown		TRIAL PIT NO. TP08	
LOGGED BY EK		SHEET Sheet 1 of 1	
CO-ORDINATES 712,455.10 E 726,802.32 N		DATE STARTED 07/08/2018	
GROUND LEVEL (m) 84.73		DATE COMPLETED 07/08/2018	
CLIENT ENGINEER DBFL Consulting Engineers		EXCAVATION METHOD	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
	Firm to stiff brown slightly gravelly CLAY with a medium cobble content and pockets of sand and gravel which are less than 10mm. Cobbles are sub-angular to sub-rounded and less than 300mm in size.		0.30	84.43						
1.0						AA81278	B	1.00		
2.0										
	End of Trial Pit at 2.50m		2.50	82.23						
3.0						AA81279	B	2.00		
4.0										

Groundwater Conditions

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL_GDT_15/8/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown

TRIAL PIT NO. TP09
SHEET Sheet 1 of 1

LOGGED BY EK

CO-ORDINATES 712,394.71 E
726,888.26 N

DATE STARTED 07/08/2018
DATE COMPLETED 07/08/2018

CLIENT ENGINEER DBFL Consulting Engineers

GROUND LEVEL (m) 85.56

EXCAVATION METHOD

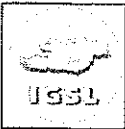
Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
	Firm to stiff brown slightly gravelly CLAY with a low cobble content. Cobbles are angular to sub-angular and less than 300mm.		0.30	85.26						
1.0						AA81271	B	1.10		
2.0										
	End of Trial Pit at 2.50m		2.50	83.06						
3.0						AA81272	B	2.10		
4.0										

Groundwater Conditions

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL_GDT 15/08/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown		TRIAL PIT NO. TP10	
LOGGED BY EK		SHEET Sheet 1 of 1	
CLIENT ENGINEER DBFL Consulting Engineers		DATE STARTED 07/08/2018	
CO-ORDINATES 712,483.42 E 726,850.01 N		DATE COMPLETED 07/08/2018	
GROUND LEVEL (m) 84.38		EXCAVATION METHOD	

Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
			Sample Ref	Type	Depth		
0.0							
0.30	84.08		AA81286	B	0.30		
			AA81287	B	0.50		
1.0			AA81288	B	1.00		
2.0			AA81289	B	2.00		
2.50	81.88						

Groundwater Conditions

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL GDT 15/8/18



TRIAL PIT RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown	TRIAL PIT NO. TP11
	SHEET Sheet 1 of 1
LOGGED BY EK	CO-ORDINATES 712,498.97 E 726,906.05 N
	DATE STARTED 07/08/2018
	DATE COMPLETED 07/08/2018
CLIENT ENGINEER DBFL Consulting Engineers	GROUND LEVEL (m) 83.22
	EXCAVATION METHOD

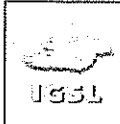
Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (kPa)	Hand Penetrometer (kPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
	Firm to stiff brown gravelly CLAY with a low cobble content. Cobbles are sub-angular to sub-rounded and less than 200mm in size.		0.30	82.92						
1.0						AA81282	B	1.00		
2.0	End of Trial Pit at 2.10m		2.10	81.12		AA81283	B	2.00		
3.0										
4.0										

Groundwater Conditions

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL.GDT 15/8/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown	TRIAL PIT NO. TP12
	SHEET Sheet 1 of 1
LOGGED BY K. Kinsella	CO-ORDINATES 712,427.12 E 726,940.30 N
	DATE STARTED 10/08/2018
	DATE COMPLETED 10/08/2018
CLIENT ENGINEER DBFL Consulting Engineers	GROUND LEVEL (m) 83.88
	EXCAVATION METHOD

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	Firm brown TOPSOIL with rootlets									
	Firm brown/light brown sandy SILT/CLAY with occasional subangular to subrounded gravel		0.15	83.73						
	Firm to stiff brown sandy gravelly SILT/CLAY with occasional subrounded boulders up to 450mm, cobble content increases with depth		0.55	83.33		AA98926	B	0.40-0.50		
1.0						AA98927	B	1.00-1.10		
2.0						AA98928	B	2.00-2.10		
2.80	End of Trial Pit at 2.80m		2.80	81.08						

Groundwater Conditions
Dry

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL.GDT 15/8/18



TRIAL PIT RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown

TRIAL PIT NO. TP13
SHEET Sheet 1 of 1

LOGGED BY EK

CO-ORDINATES 712,427.12 E
726,847.94 NDATE STARTED 07/08/2018
DATE COMPLETED 07/08/2018

CLIENT ENGINEER DBFL Consulting Engineers

GROUND LEVEL (m) 85.23

EXCAVATION METHOD





Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
0.30	Firm to stiff brown gravelly CLAY with a low cobble content. Cobbles are sub-angular to sub-rounded and less than 300mm in size.		0.30	84.93						
1.0						AA81280	B	1.00		
2.0						AA81281	B	2.00		
2.60	End of Trial Pit at 2.60m		2.60	82.63						

Groundwater Conditions

Stability
GoodGeneral Remarks
Pit terminated due to very slow progress

 IGSL	TRIAL PIT RECORD	REPORT NUMBER 21167
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CONTRACT Beech House, Scholarstown	CO-ORDINATES 712,513.34 E 726,975.55 N	TRIAL PIT NO. TP14 SHEET Sheet 1 of 1	DATE STARTED 10/08/2018 DATE COMPLETED 10/08/2018
LOGGED BY K. Kinsella	GROUND LEVEL (m) 81.03	EXCAVATION METHOD	
CLIENT ENGINEER DBFL Consulting Engineers			

	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	Firm brown TOPSOIL with rootlets		0.10	80.93						
	Firm brown sandy SILT/CLAY with occasional subangular to subrounded gravel					AA98923	B	0.50-0.60		
1.0	Stiff to firm brown sandy gravelly slightly cobbly SILT/CLAY with rare subangular to subrounded boulders up to 450mm, cobble content increases with depth.		1.05	79.98		AA98924	B	1.50-1.60		
2.0						AA98925	B	2.50-2.60		
3.0	End of Trial Pit at 2.75m		2.75	78.28						
4.0										

Groundwater Conditions
Dry


Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG 21167.GPJ IGSL_GDT 15/08/18

 IGSL	TRIAL PIT RECORD	REPORT NUMBER 21167
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CONTRACT Beech House, Scholarstown	TRIAL PIT NO. TP15	SHEET Sheet 1 of 1
LOGGED BY EK	CO-ORDINATES 712,528.38 E 726,918.79 N	DATE STARTED 07/08/2018
CLIENT ENGINEER DBFL Consulting Engineers	GROUND LEVEL (m) 82.23	DATE COMPLETED 07/08/2018
EXCAVATION METHOD		

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation	Water Strike	Samples			Vane Test (KPa)	Hand Penetrometer (KPa)
						Sample Ref	Type	Depth		
0.0	TOPSOIL									
	Firm to stiff brown slightly gravelly SILT/CLAY with a medium cobble content and a low boulder content. Cobbles and boulders are sub-angular to sub-rounded and less than 400mm in size.		0.30	81.93						
1.0						AA81284	B	1.00		
2.0						AA81285	B	2.00		
2.30	End of Trial Pit at 2.30m		2.30	79.93						

Groundwater Conditions

Stability
Good

General Remarks
Pit terminated due to very slow progress

IGSL TP LOG_21167.GPJ IGSL_GDT_15/8/18

TP01 - 1 of 2



TP01 - 2 of 2



TP02 - 1 of 2



TP02 - 2 of 2



TP03 - 1 of 2



TP03 - 2 of 2



TP04 - 1 of 2



TP04 - 2 of 2



TP05 - 1 of 2



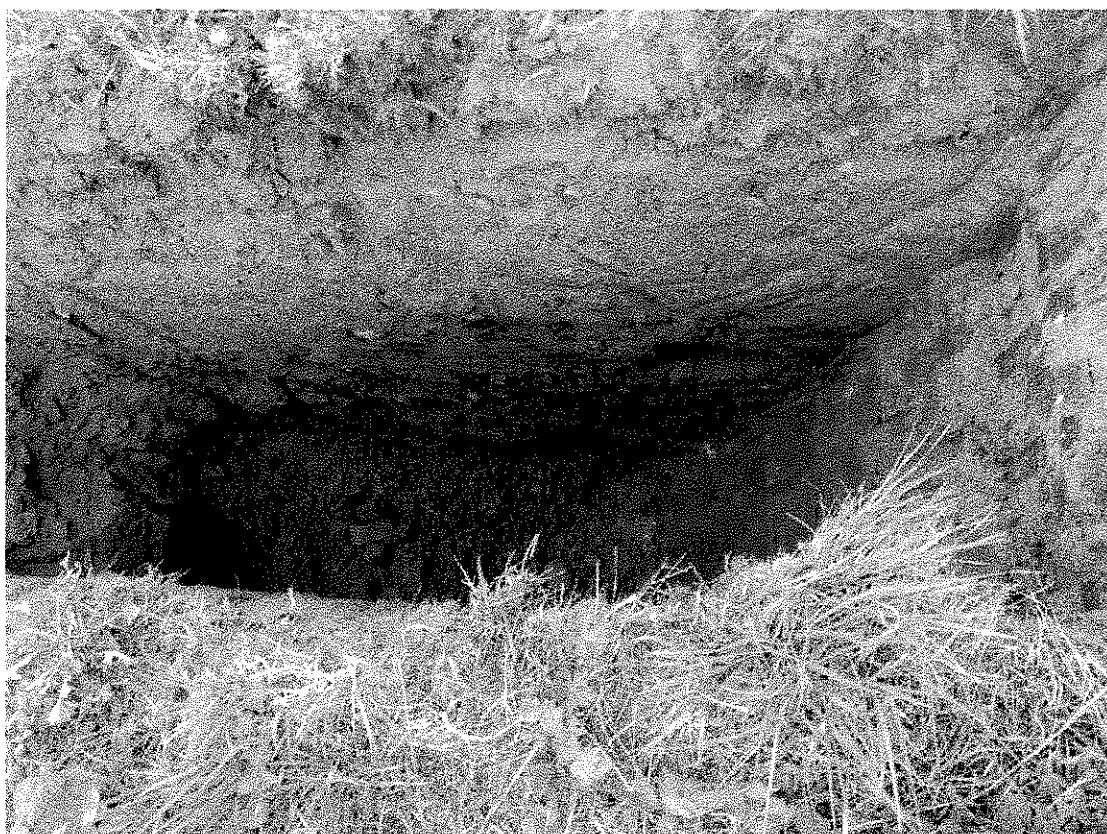
TP05 - 2 of 2



TP06 - 1 of 2



TP06 - 2 of 2



TP07 - 1 of 2



TP07 - 2 of 2



TP08 - 1 of 2



TP08 - 2 of 2



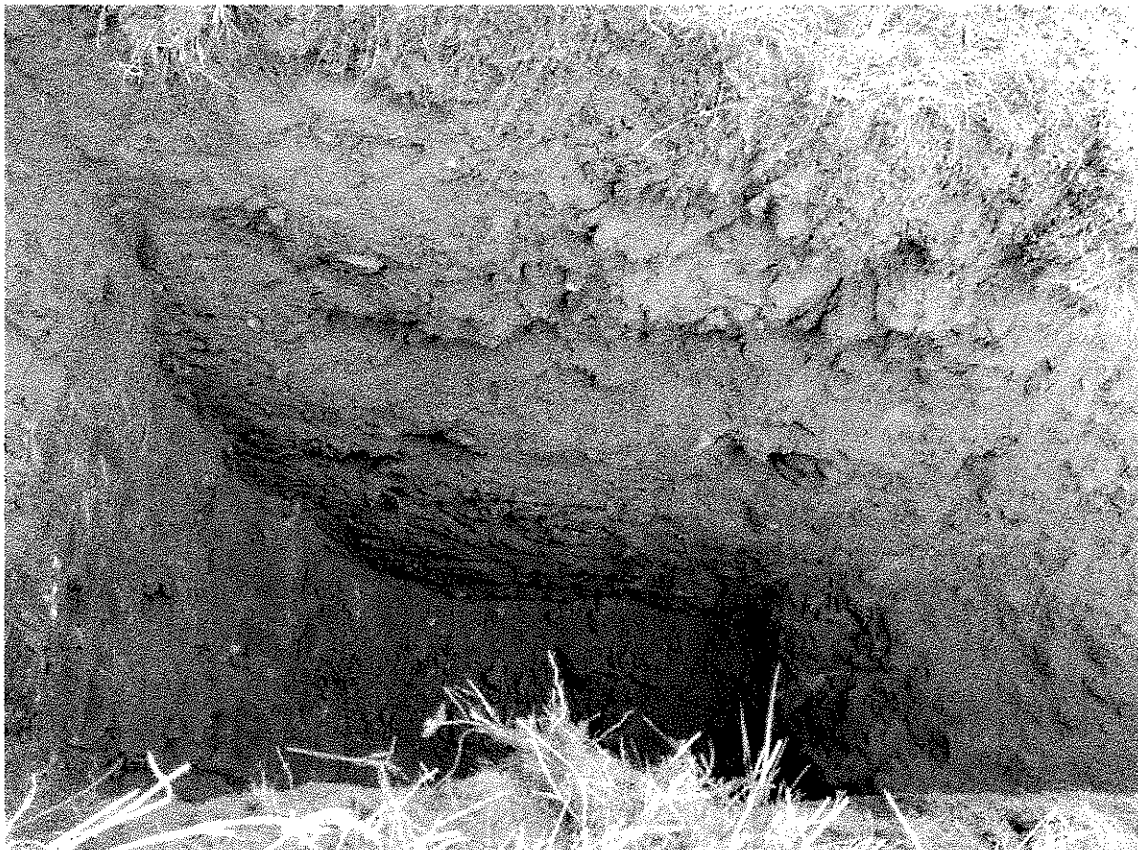
TP09 - 1 of 1



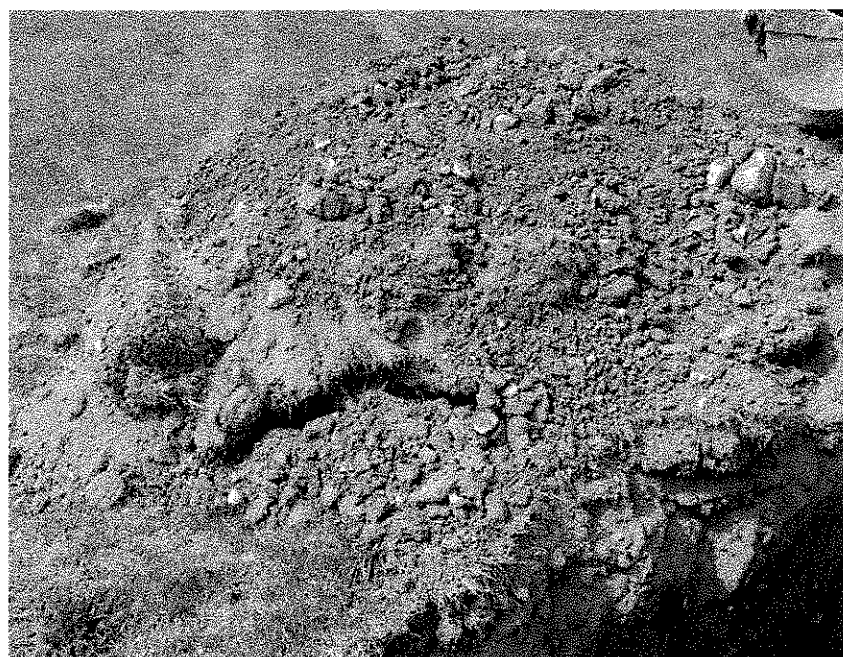
TP10 - 1 of 2



TP10 - 2 of 2



TP11 - 1 of 2



TP11 - 2 of 2



TP12 - 1 of 2



TP12 - 2 of 2



TP13 - 1 of 2



TP13 - 2 of 2



TP14 - 1 of 2



TP14 - 2 of 2



TP15 - 1 of 2



TP15 - 2 of 2



Appendix III Plate Bearing Tests

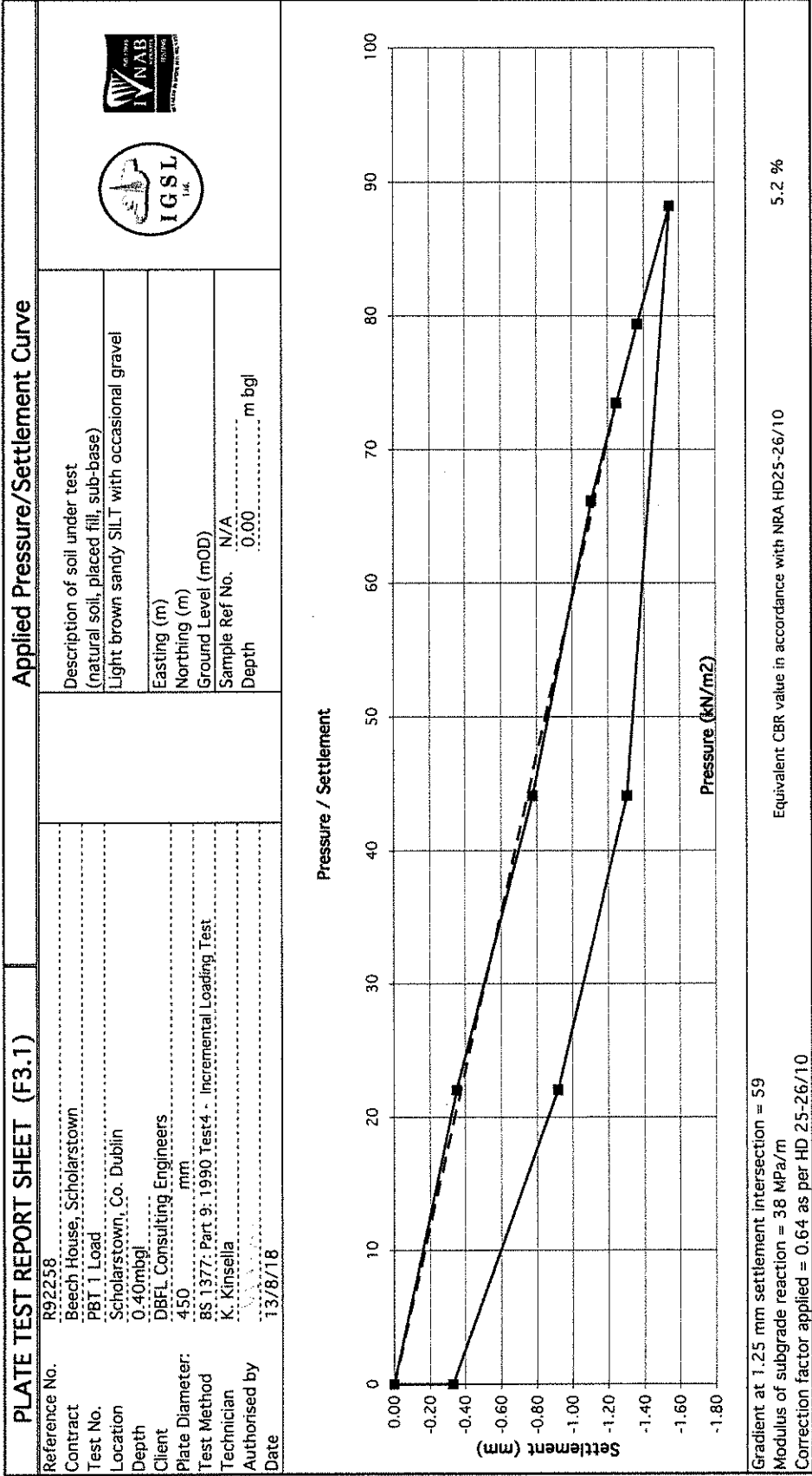


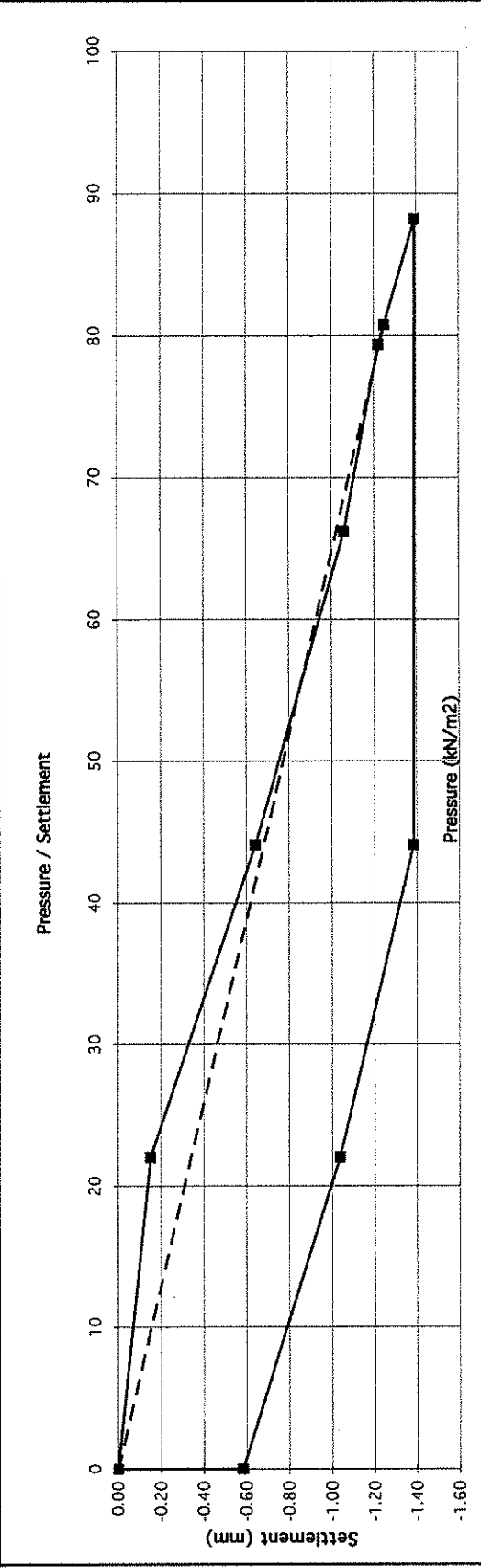


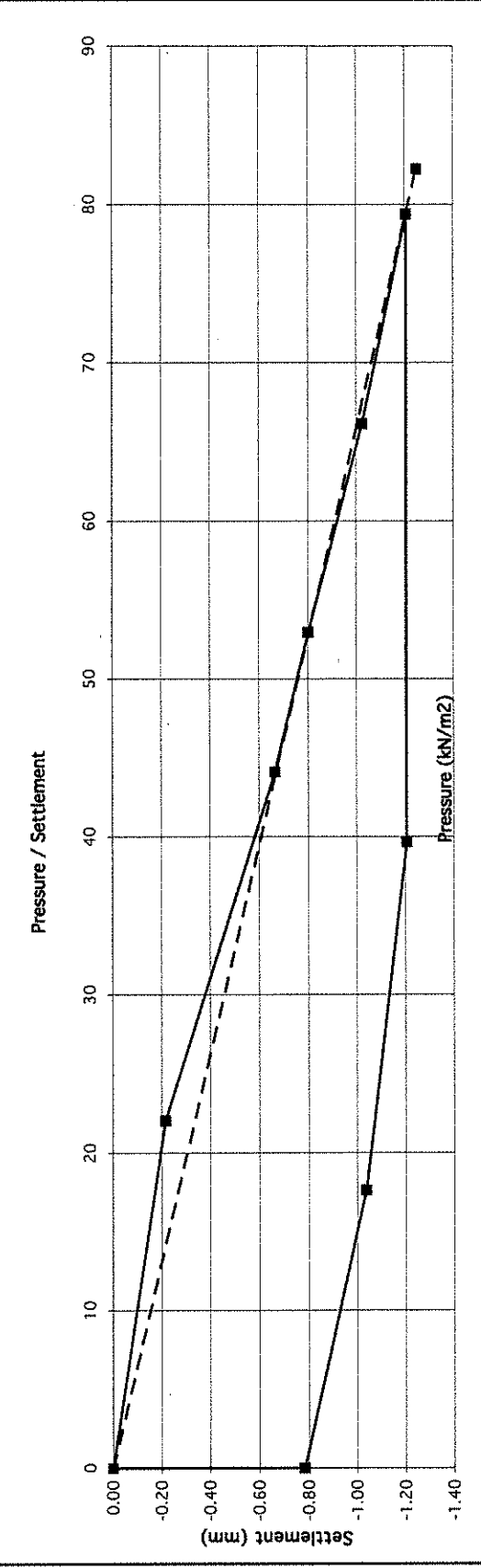
PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92258	Description of soil under test (natural soil, placed fill, sub-base)	 
Contract	Beech House, Scholarstown	Light brown sandy SILT with occasional gravel	
Test No.	PBT 1 ReLoad		
Location	Scholarstown, Co. Dublin	Easting (m)	
Depth	0.40mbgl	Northing (m)	
Client	DBFL Consulting Engineers	Ground Level (mOD)	
Plate Diameter:	450 mm	Sample Ref No.	N/A
Test Method	BS 1377: Part 9: 1990 Test 4 - Incremental Loading Test	Depth	0.00 m bgl
Technician	K. Kinsella		
Authorised by			
Date	13/8/18		



Gradient at 1.25 mm settlement intersection = 65
 Modulus of subgrade reaction = 42 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10

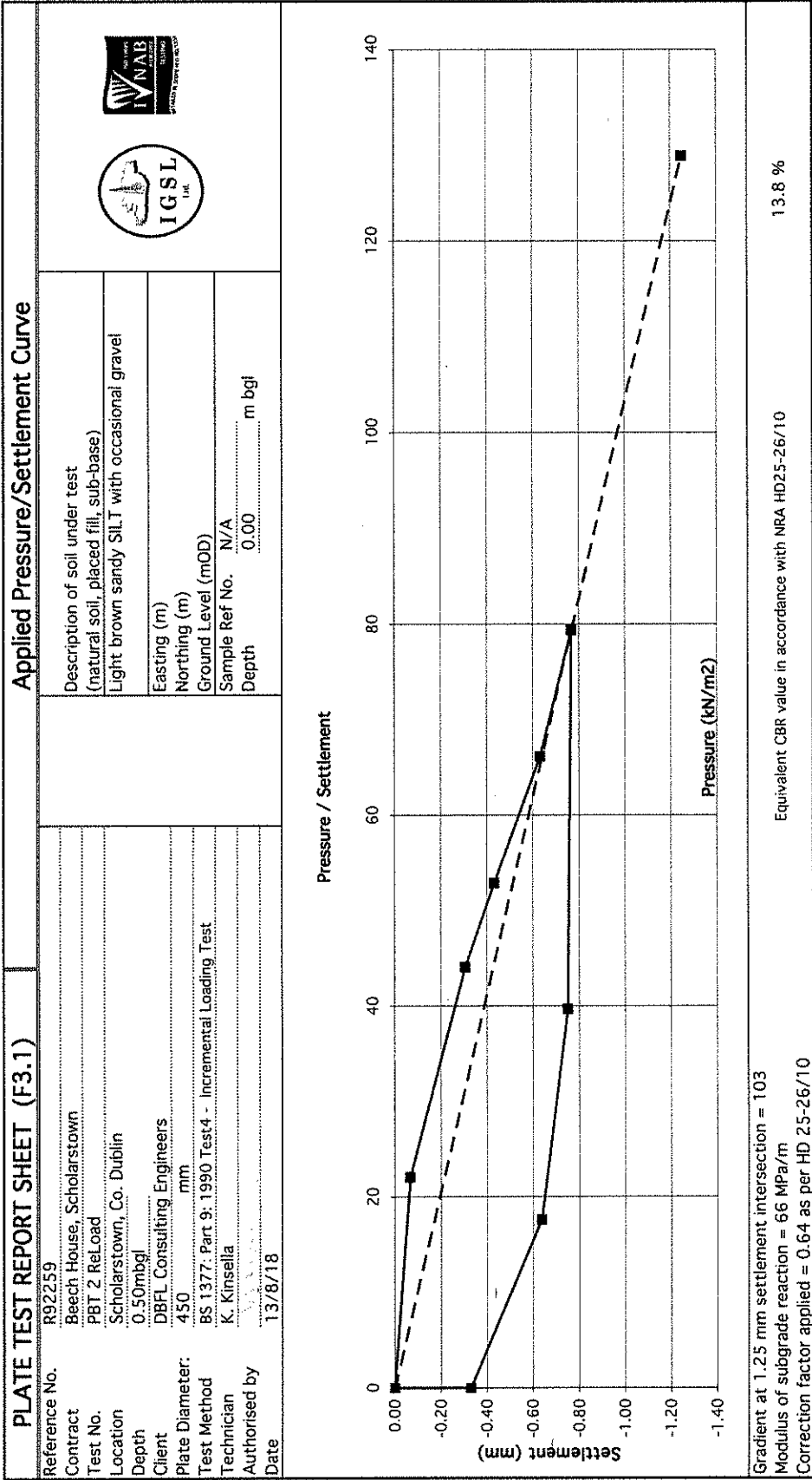
Equivalent CBR value in accordance with NRA HD25-26/10 **6.2 %**

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92259	Description of soil under test (natural soil, placed fill, sub-base)	Light brown sandy SILT with occasional gravel
Contract	Beech House, Scholarstown		
Test No.	PBT 2 Load	Easting (m)	
Location	Scholarstown, Co. Dublin	Northing (m)	
Depth	0.50mbgl	Ground Level (mOD)	
Client	DBFL Consulting Engineers	Sample Ref No.	N/A
Plate Diameter:	450 mm	Depth	0.00 m bgl
Test Method	BS 1377: Part 9: 1990 Test 4 - Incremental Loading Test		
Technician	K. Kinsella		
Authorised by			
Date	13/8/18		



Gradient at 1.25 mm settlement intersection = 66
 Modulus of subgrade reaction = 42 MPa/m
 (Correction factor applied = 0.64 as per HD 25-26/10)

Equivalent CBR value in accordance with NRA HD25-26/10
 6.4 %



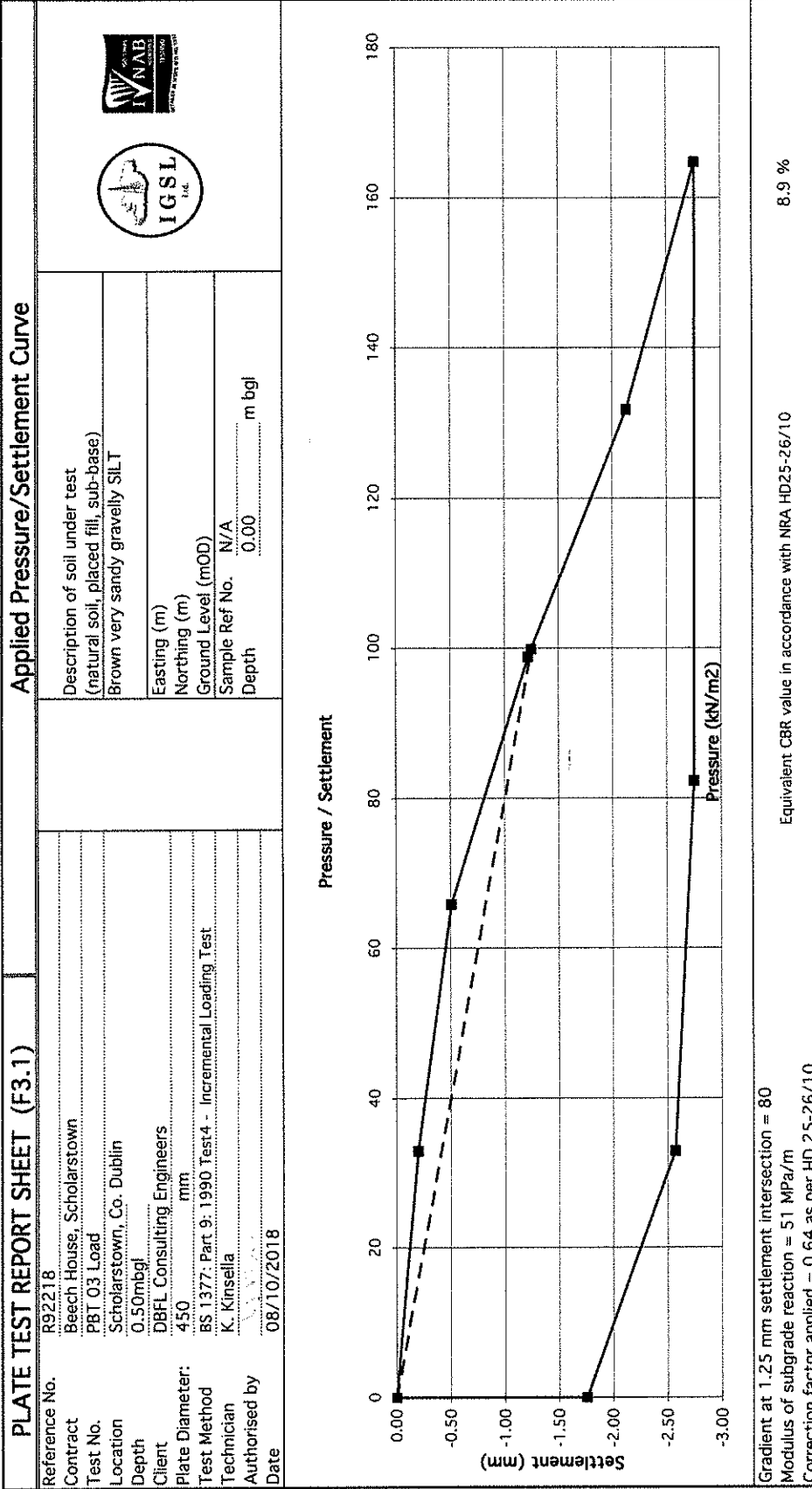


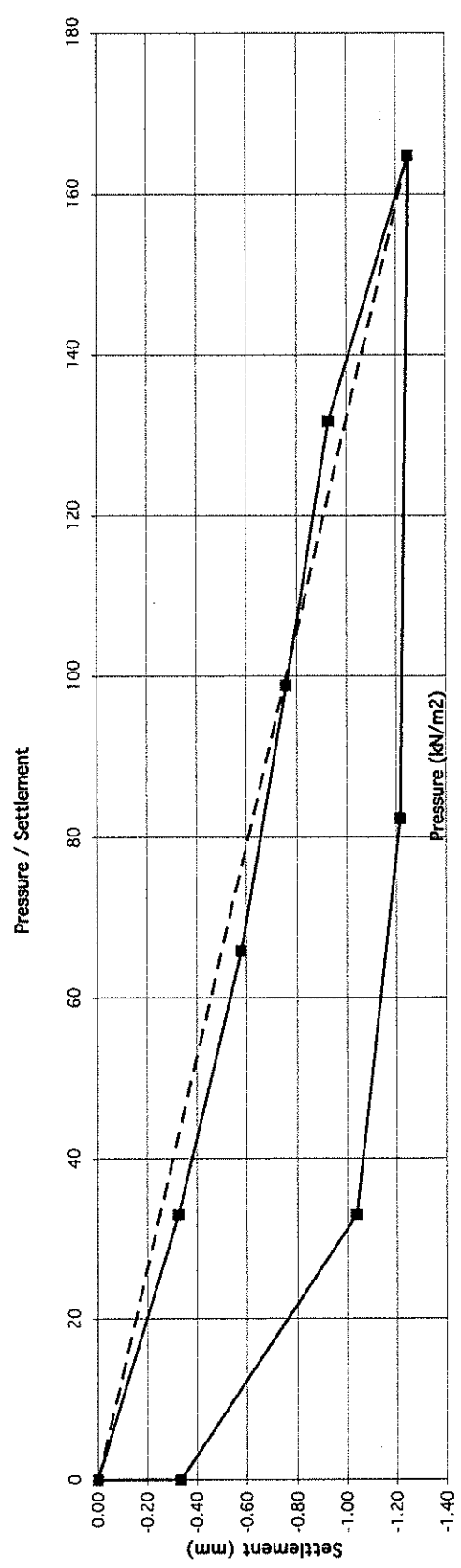




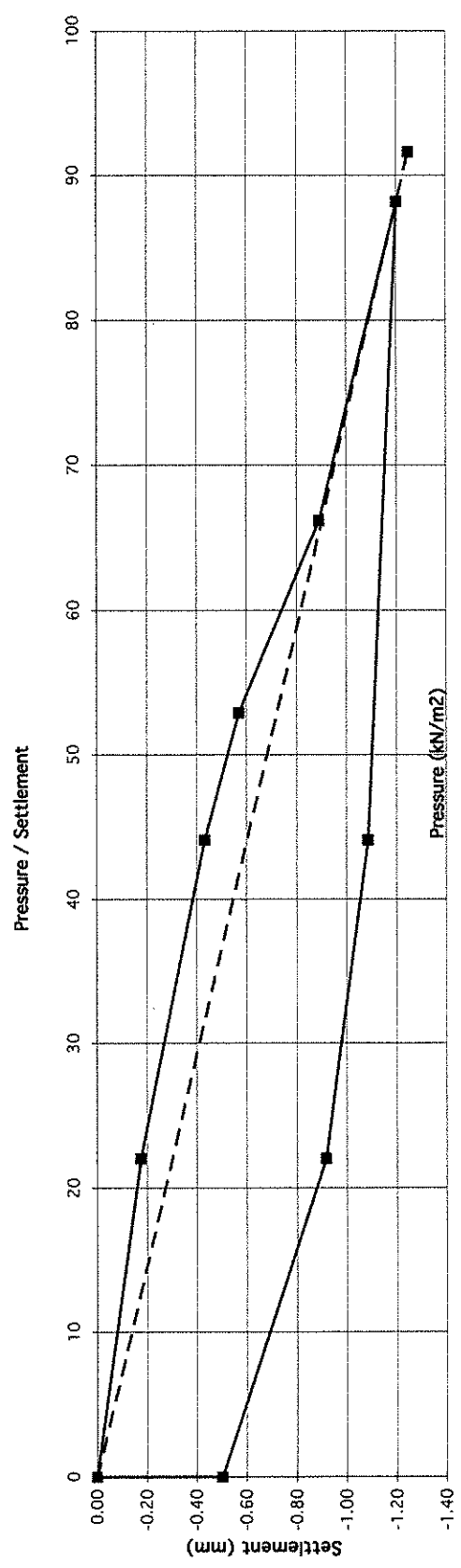
PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92218	Description of soil under test (natural soil, placed fill, sub-base)	 
Contract	Beech House, Scholarstown		
Test No.	PBT 03 ReLoad	Brown very sandy gravelly SILT	Easting (m)
Location	Scholarstown, Co. Dublin		
Depth	0.50mbgl	Ground Level (mOD)	0.00
Client	DBFL Consulting Engineers	Sample Ref No.	N/A
Plate Diameter:	450 mm	Depth	0.00 m bgl
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test		
Technician	K. Kinsella		
Authorised by			
Date	08/10/2018		



Gradient at 1.25 mm settlement intersection = 132
 Modulus of subgrade reaction = 85 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10

Equivalent CBR value in accordance with NRA HD25-26/10
 2.1.2 %

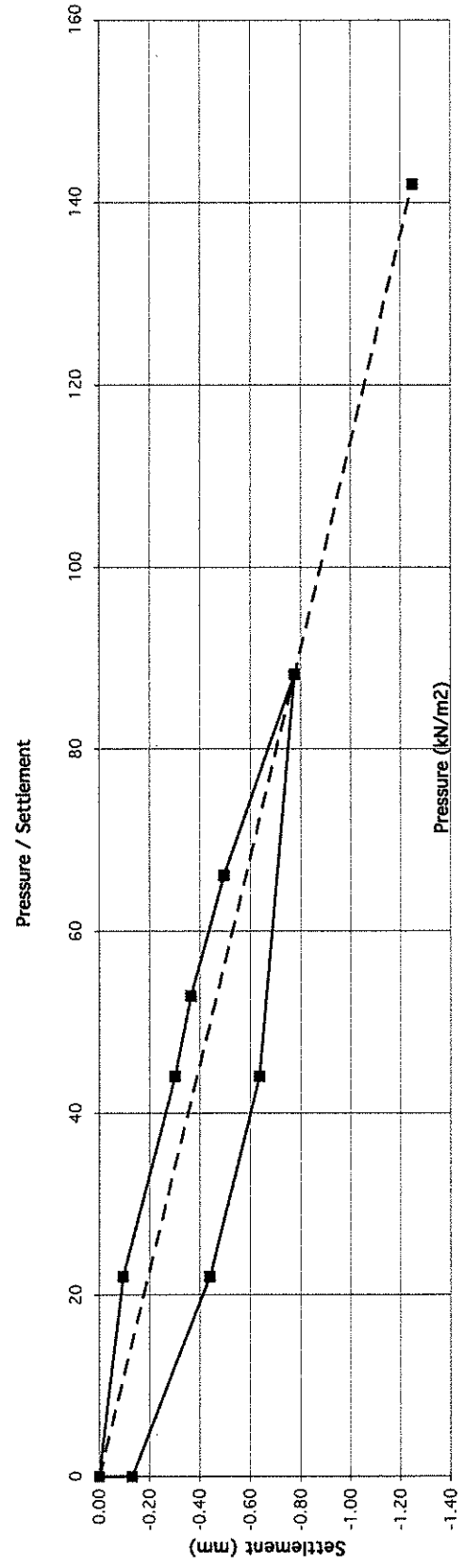
PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92260	Description of soil under test (natural soil, placed fill, sub-base)	 
Contract	Beech House, Scholarstown		
Test No.	PBT 4 Load	Light brown sandy SILT	
Location	Scholarstown, Co. Dublin		
Depth	0.50mbgl		
Client	DBFL Consulting Engineers		
Plate Diameter:	450 mm		
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test		
Technician	K. Kinsella		
Authorised by			
Date	13/8/18		
		Eastings (m)	
		Northing (m)	
		Ground Level (mOD)	
		Sample Ref No.	N/A
		Depth	0.00 m bgl



Gradient at 1.25 mm settlement intersection = 73
 Modulus of subgrade reaction = 47 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10



Equivalent CBR value in accordance with NRA HD25-26/10 7.7 %

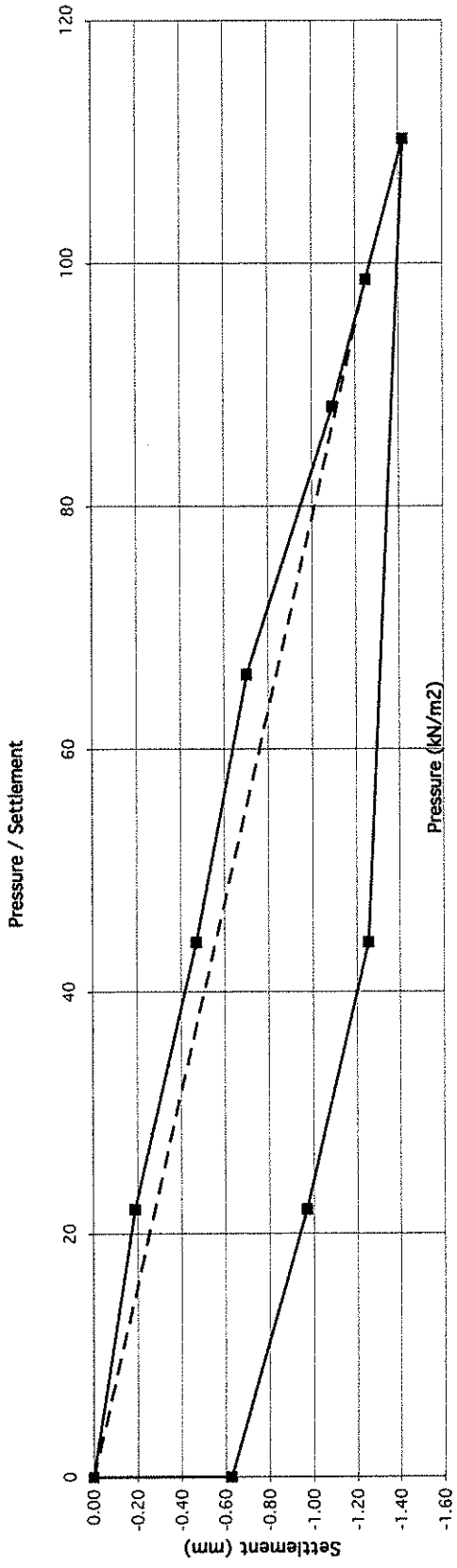
PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92260	Description of soil under test (natural soil, placed fill, sub-base)	Light brown sandy SILT
Contract	Beech House, Scholarstown		
Test No.	PBT 4 ReLoad	Easting (m)	
Location	Scholarstown, Co. Dublin	Northing (m)	
Depth	0.50mbgl	Ground Level (mOD)	
Client	DBFL Consulting Engineers	Sample Ref No.	N/A
Plate Diameter:	450 mm	Depth	0.00 m bgl
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test		
Technician	K. Kinsella		
Authorised by			
Date	13/8/18		



Gradient at 1.25 mm settlement intersection = 114
 Modulus of subgrade reaction = 73 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10

Equivalent CBR value in accordance with NRA HD25-26/10 16.4 %



PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No. R92261	Beech House, Scholarstown	Description of soil under test (natural soil, placed fill, sub-base) Light brown sandy slightly gravelly SILT with rare cobbles	 
Contract PBT 5 Load	Scholarstown, Co. Dublin		
Test No.	0.50mbgl	Easting (m)	
Location	DBFL Consulting Engineers	Northing (m)	
Depth	450 mm	Ground Level (mOD)	
Client	85 1377 - Part 9 - 1990 Test4 - Incremental Loading Test	Sample Ref No.	N/A
Plate Diameter:	K. Kinsella	Depth	0.00 m bgl
Test Method			
Technician			
Authorised by			
Date	13/8/18		

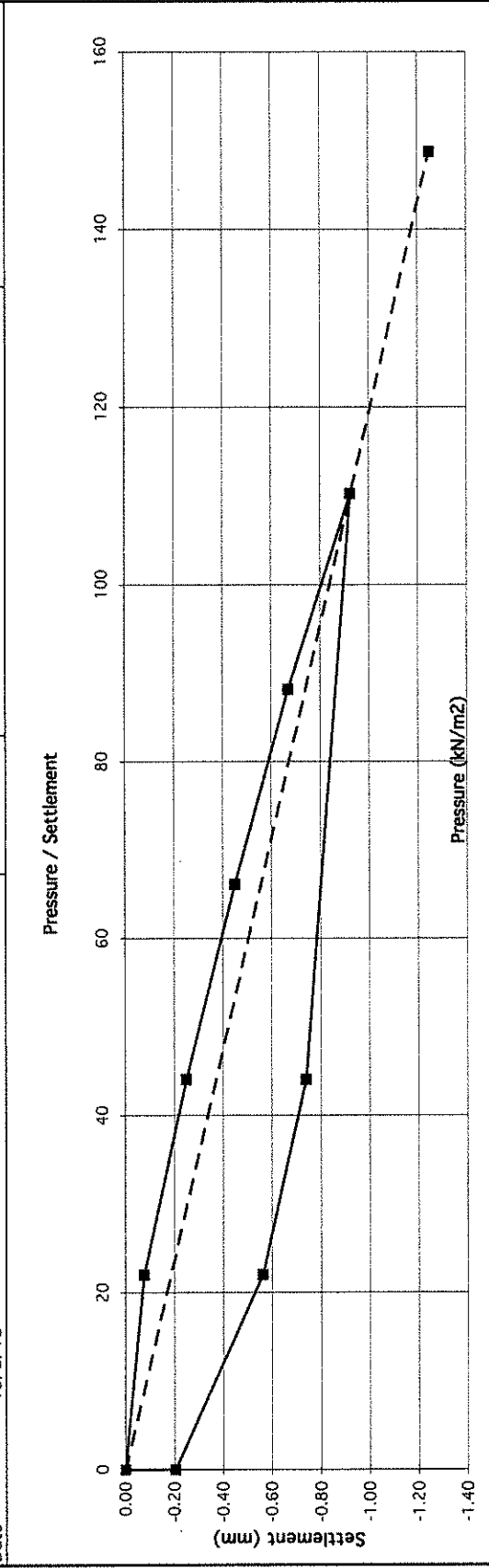


Gradient at 1.25 mm settlement intersection = 79
 Modulus of subgrade reaction = 51 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10



Equivalent CBR value in accordance with NRA HD25-26/10

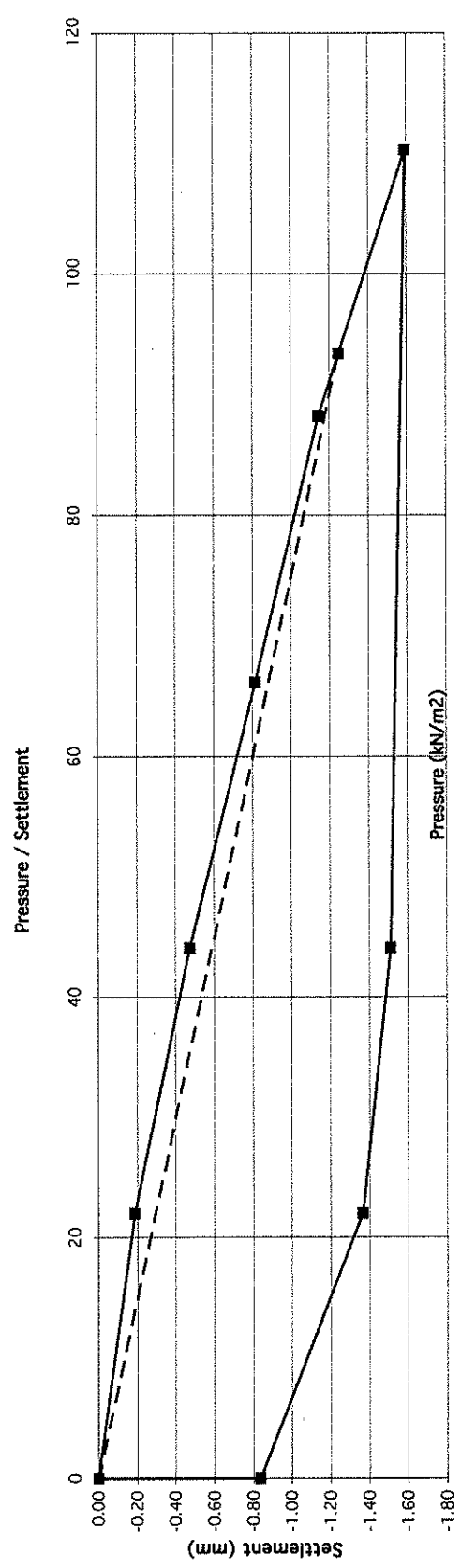
8.7 %

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92261	Description of soil under test (natural soil, placed fill, sub-base)	 
Contract	Beech House, Scholarstown	Light brown sandy slightly gravelly SILT with rare cobbles	
Test No.	PBT 5 ReLoad	Easting (m)	
Location	Scholarstown, Co. Dublin	Northing (m)	
Depth	0.50mbgl	Ground Level (mOD)	
Client	DBFL Consulting Engineers	Sample Ref No.	N/A
Plate Diameter:	450 mm	Depth	0.00 m bgl
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test		
Technician	K. Kinsella		
Authorised by			
Date	13/8/18		





Gradient at 1.25 mm settlement intersection = 119
 Modulus of subgrade reaction = 77 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10
 Equivalent CBR value in accordance with NRA HD25-26/10 17.7 %

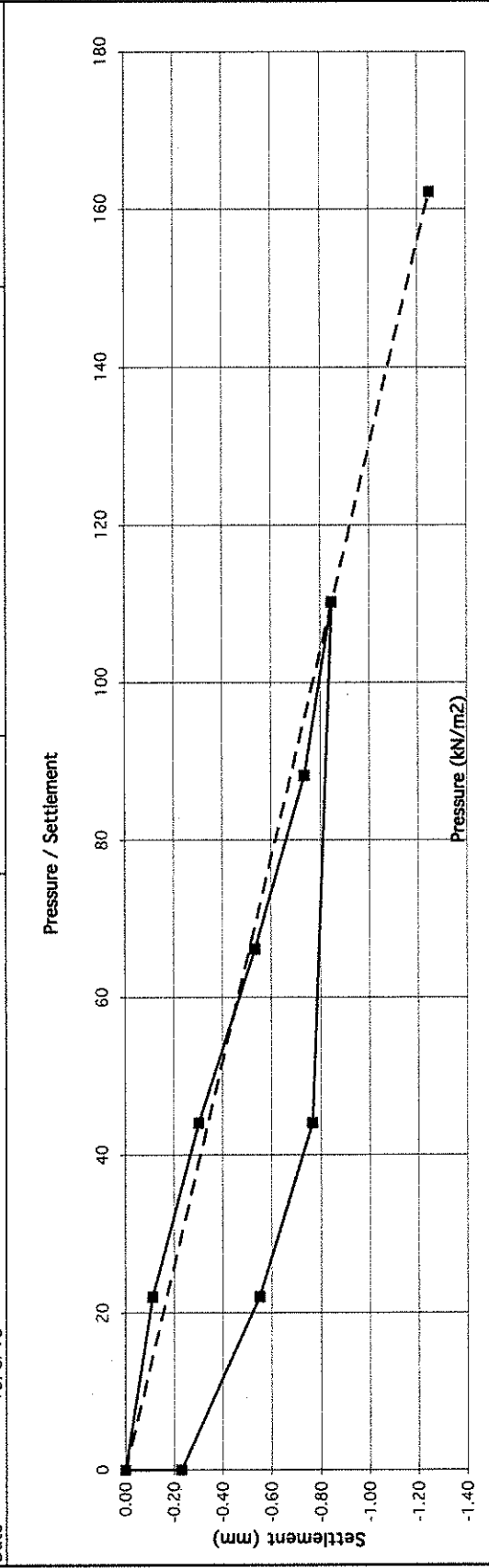
PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No. R92262	Beech House, Scholarstown	Description of soil under test (natural soil, placed fill, sub-base) Light brown sandy slightly gravelly SILT with rare cobbles	 
Contract PBT 6 Load	Scholarstown, Co. Dublin		
Location Scholarstown, Co. Dublin	0.50mbgl	Easting (m)	
Depth 0.50mbgl	DBFL Consulting Engineers	Northing (m)	
Client DBFL Consulting Engineers	450 mm	Ground Level (mOD)	
Plate Diameter: 450 mm	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	Sample Ref No.	N/A
Test Method BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	K. Kinsella	Depth	0.00 m bgl
Technician K. Kinsella			
Authorised by			
Date 13/8/18			





Gradient at 1.25 mm settlement intersection = 75
 Modulus of subgrade reaction = 48 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10
 Equivalent CBR value in accordance with NRA HD25-26/10
 7.9 %

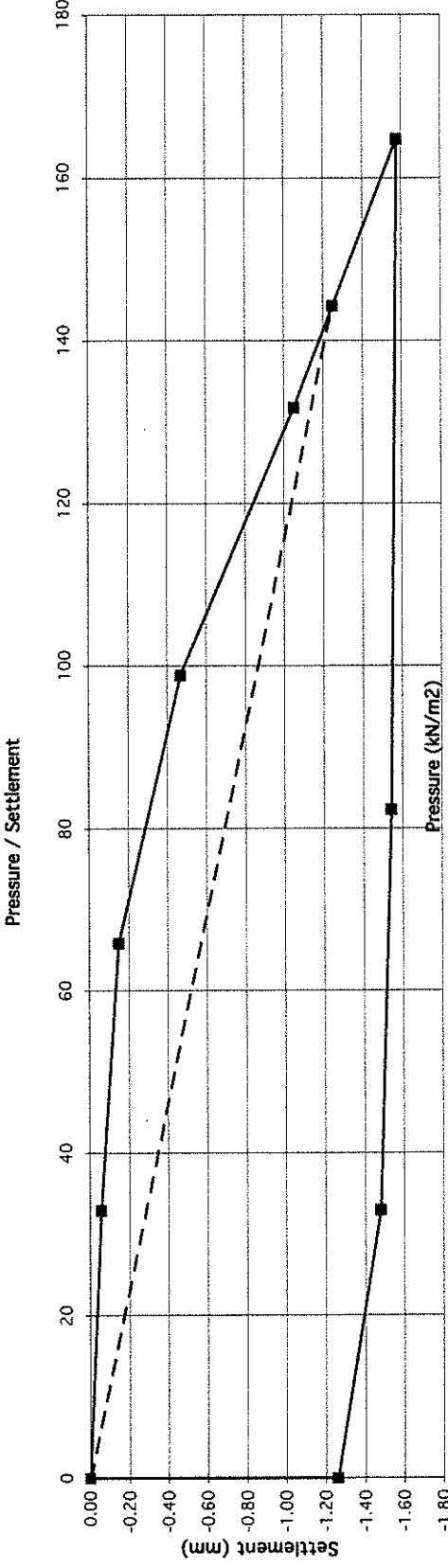
PLATE TEST REPORT SHEET (F3.1)

Reference No. R92262	Beech House, Scholarstown	 
Contract PBT 6 Reload	Scholarstown, Co. Dublin	
Test No.	0.50mbgl	Description of soil under test (natural soil, placed fill, sub-base) Light brown sandy slightly gravelly SILT with rare cobbles
Location	DBFL Consulting Engineers	
Depth	450 mm	Easting (m)
Client	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	Northing (m)
Plate Diameter:	K. Kinsella	Ground Level (mOD)
Test Method	13/8/18	Sample Ref No. N/A
Technician		Depth 0.00 m bgl
Authorised by		
Date		



Gradient at 1.25 mm settlement intersection = 130
 Modulus of subgrade reaction = 83 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10
 Equivalent CBR value in accordance with NRA HD25-26/10 20.6 %

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No. R92219	Contract Beech House, Scholarstown	Description of soil under test (natural soil, placed fill, sub-base) Brown sandy slightly gravelly SIL.T	 
Test No. PBT 07 Load	Location Scholarstown, Co. Dublin	Easting (m)	
Depth 0.50mbgl	Client DBFL Consulting Engineers	Northing (m)	
Plate Diameter: 450 mm	Test Method BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	Ground Level (mOD)	
Technician K. Kinsella	Authorised by	Sample Ref No. N/A	
Date 08/10/2018		Depth 0.00 m bgl	



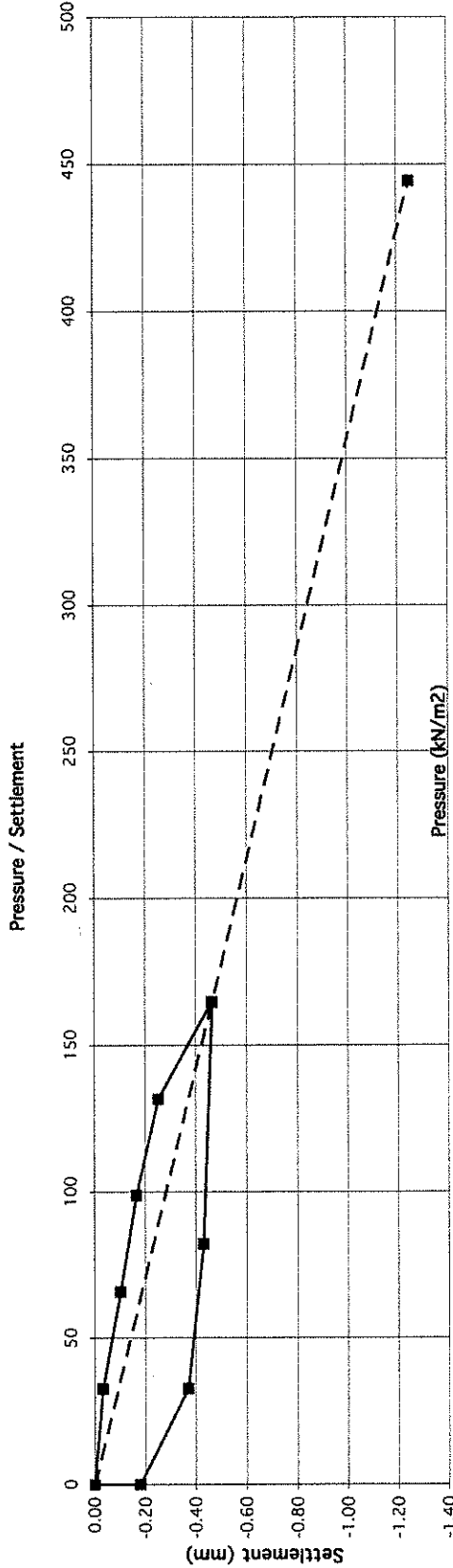
The graph plots Settlement (mm) on the y-axis (from 0.00 to -1.80) against Pressure / Settlement (kN/m²) on the x-axis (from 0 to 180). A solid line with square markers shows the test data, and a dashed line shows the theoretical curve. The test data points are approximately: (0, 0), (10, -0.15), (20, -0.25), (30, -0.35), (40, -0.45), (50, -0.55), (60, -0.65), (70, -0.75), (80, -0.85), (90, -0.95), (100, -1.05), (110, -1.15), (120, -1.25), (130, -1.35), (140, -1.45), (150, -1.55), (160, -1.65), (170, -1.75).

Gradient at 1.25 mm settlement intersection = 115
Modulus of subgrade reaction = 74 MPa/m
Correction factor applied = 0.64 as per HD 25-26/10

Equivalent CBR value in accordance with NRA HD25-26/10 16.8 %

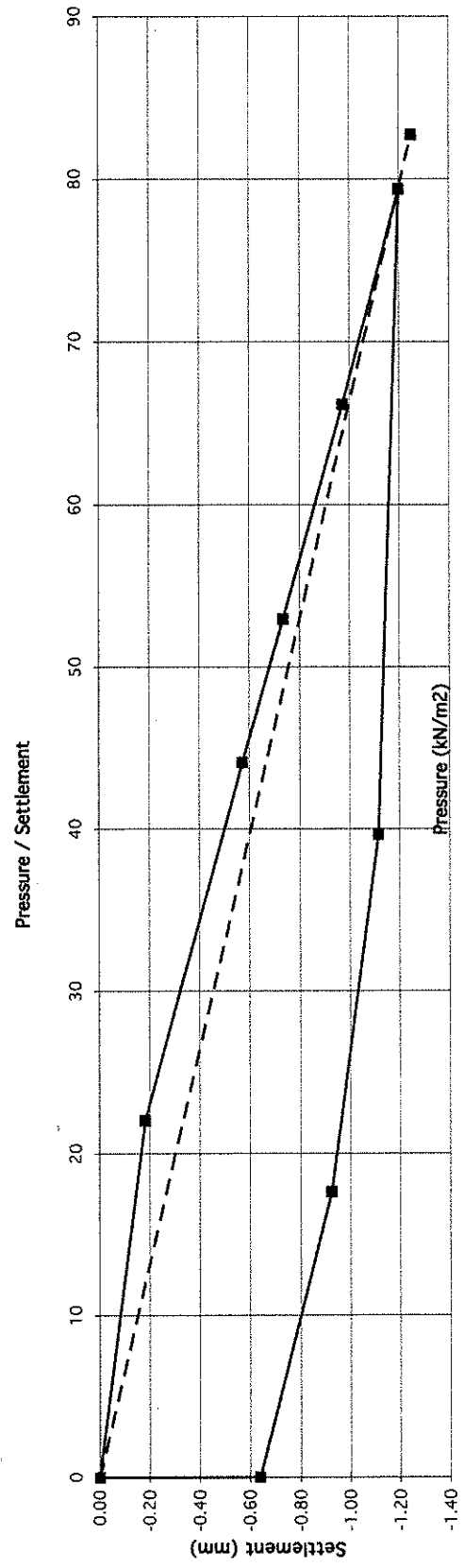
PLATE TEST REPORT SHEET (F3.1) **Applied Pressure/Settlement Curve**

Reference No. R92219 Contract Beech House, Scholarstown Test No. PBT 07 ReLoad Location Scholarstown, Co. Dublin Depth 0.50mbgl Client DBFL Consulting Engineers Plate Diameter: 450 mm Test Method BS 1377: Part 9: 1990 Test4 - Incremental Loading Test Technician K. Kinsella Authorised by Date 08/10/2018	Description of soil under test (natural soil, placed fill, sub-base) Brown sandy slightly gravely SILT Easting (m) Northing (m) Ground Level (mOD) Sample Ref No. N/A Depth 0.00 m bgl
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Gradient at 1.25 mm settlement intersection = 356
 Modulus of subgrade reaction = 229 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10
 Equivalent CBR value in accordance with NRA HD25-26/10 118.2 %

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92263	Description of soil under test (natural soil, placed fill, sub-base)	Light brown sandy slightly gravelly SILT with rare cobbles
Contract	Beech House, Scholarstown		
Test No.	PBT 8 Load	Easting (m)	
Location	Scholarstown, Co. Dublin	Northing (m)	
Depth	0.50mbgl	Ground Level (mOD)	
Client	DBFL Consulting Engineers	Sample Ref No.	N/A
Plate Diameter:	450 mm	Depth	0.00 m bgl
Test Method	BS 1377: Part 9: 1990 Test 4 - Incremental Loading Test		
Technician	K. Kinsella		
Authorised by			
Date	13/8/18		



Gradient at 1.25 mm settlement intersection = 66
 Modulus of subgrade reaction = 43 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10

Equivalent CBR value in accordance with NRA HD25-26/10
 6.4 %

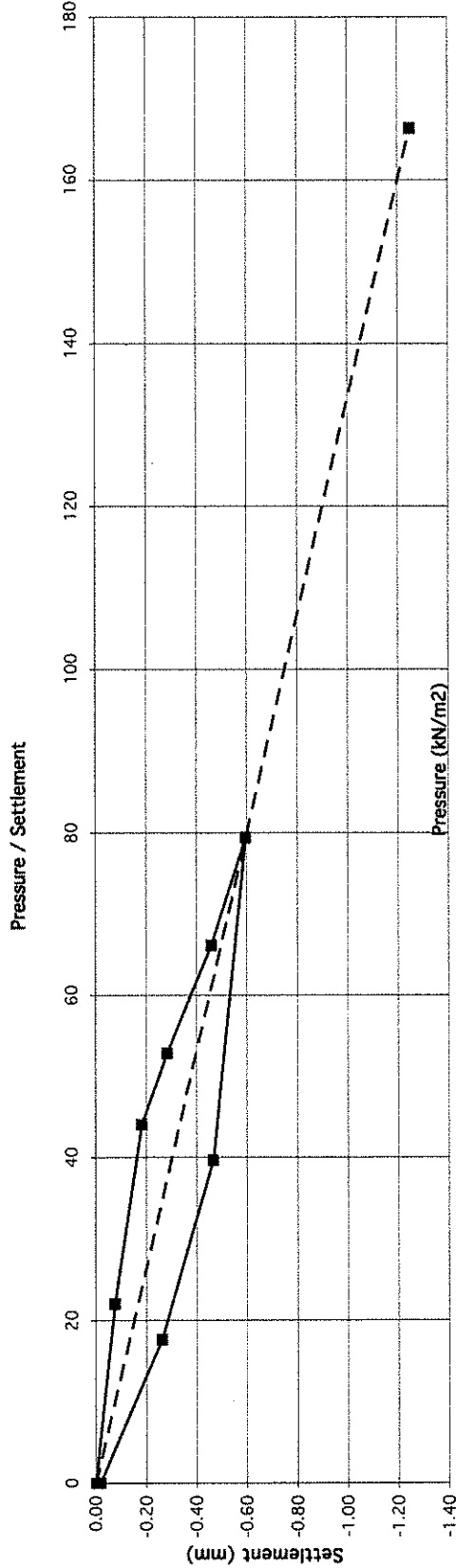
PLATE TEST REPORT SHEET (F3.1)

Reference No. R92263
 Contract Beech House, Scholarstown
 Test No. PBT 8 ReLoad
 Location Scholarstown, Co. Dublin
 Depth 0.50mbgl
 Client DBFL Consulting Engineers
 Plate Diameter: 450 mm
 Test Method BS 1377: Part 9: 1990 Test4 - Incremental Loading Test
 Technician K. Kinsella
 Authorised by
 Date 13/8/18

Applied Pressure/Settlement Curve

Description of soil under test
 (natural soil, placed fill, sub-base)
 Light brown sandy slightly gravelly SILT with rare cobbles



Easting (m)
 Northing (m)
 Ground Level (mOD)
 Sample Ref No. N/A
 Depth 0.00 m bgl

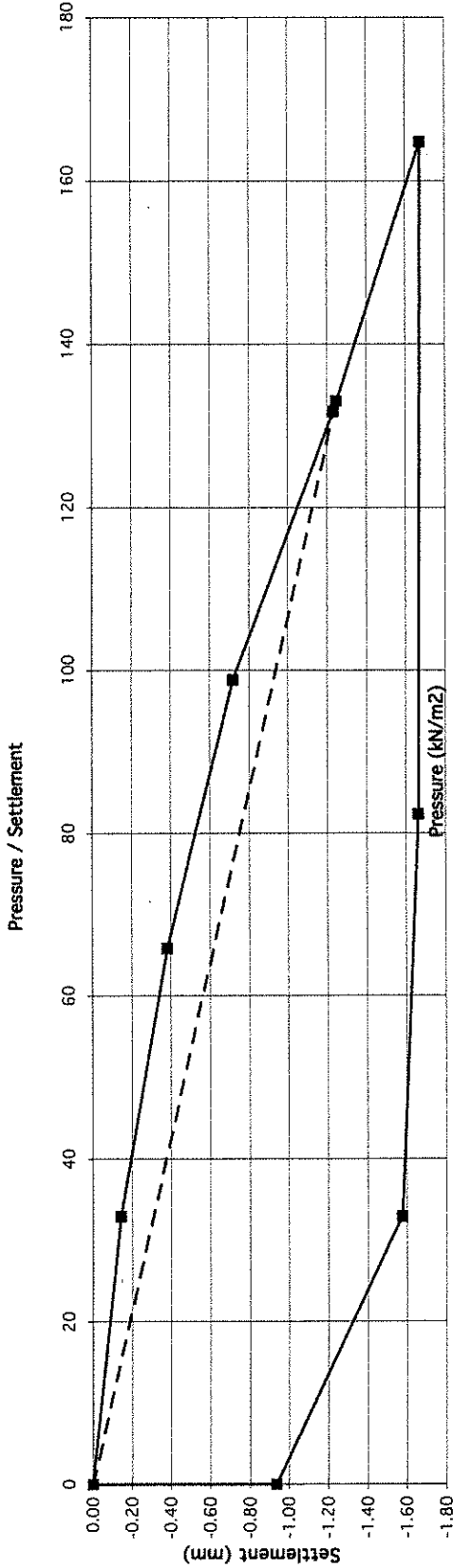


Gradient at 1.25 mm settlement intersection = 133
 Modulus of subgrade reaction = 86 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10

Equivalent CBR value in accordance with NRA HD25-26/10

21.5 %

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92220	Description of soil under test (natural soil, placed fill, sub-base)	 
Contract	Beech House, Scholarstown	Brown very sandy gravelly SILT	
Test No.	P8T 09 Load	Easting (m)	
Location	Scholarstown, Co. Dublin	Northing (m)	
Depth	0.50mbgl	Ground Level (mOD)	
Client	DBFL Consulting Engineers	Sample Ref No.	N/A
Plate Diameter:	450 mm	Depth	0.00 m bgl
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test		
Technician	K. Kinsella		
Authorised by			
Date	08/10/2018		



Pressure (kN/m ²)	Settlement (mm)
0	0.00
20	-0.25
40	-0.45
60	-0.65
80	-0.85
100	-1.05
120	-1.25
140	-1.45
160	-1.65
175	-1.75

Gradient at 1.25 mm settlement intersection = 106	Equivalent CBR value in accordance with NRA HD25-26/10	14.6 %
Modulus of subgrade reaction = 68 MPa/m		
Correction factor applied = 0.64 as per HD 25-26/10		

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92220	Description of soil under test (natural soil, placed fill, sub-base)	IGSL Ltd
Contract	Beech House, Scholarstown		
Test No.	PBT 09 ReLoad	Brown very sandy gravelly SILT	NAB
Location	Scholarstown, Co. Dublin		
Depth	0.50mbgl	Easting (m)	
Client	DBFL Consulting Engineers	Northing (m)	
Plate Diameter:	450 mm	Ground Level (mOD)	
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	Sample Ref No.	N/A
Technician	K. Kinsella	Depth	0.00 m bgl
Authorised by			
Date	08/10/2018		

Pressure / Settlement

Pressure (kN/m ²)	Settlement (mm)
0.00	0.00
10.00	-0.15
20.00	-0.25
30.00	-0.35
40.00	-0.45
50.00	-0.55
60.00	-0.65
70.00	-0.75
80.00	-0.85
90.00	-0.95
100.00	-1.05
110.00	-1.15
120.00	-1.25
130.00	-1.35
140.00	-1.45
150.00	-1.55
160.00	-1.65
170.00	-1.75
180.00	-1.85
190.00	-1.95
200.00	-2.05
210.00	-2.15
220.00	-2.25
230.00	-2.35
240.00	-2.45
250.00	-2.55

Equivalent CBR value in accordance with NRA HD25-26/10

33.9 %



Gradient at 1.25 mm settlement intersection = 173

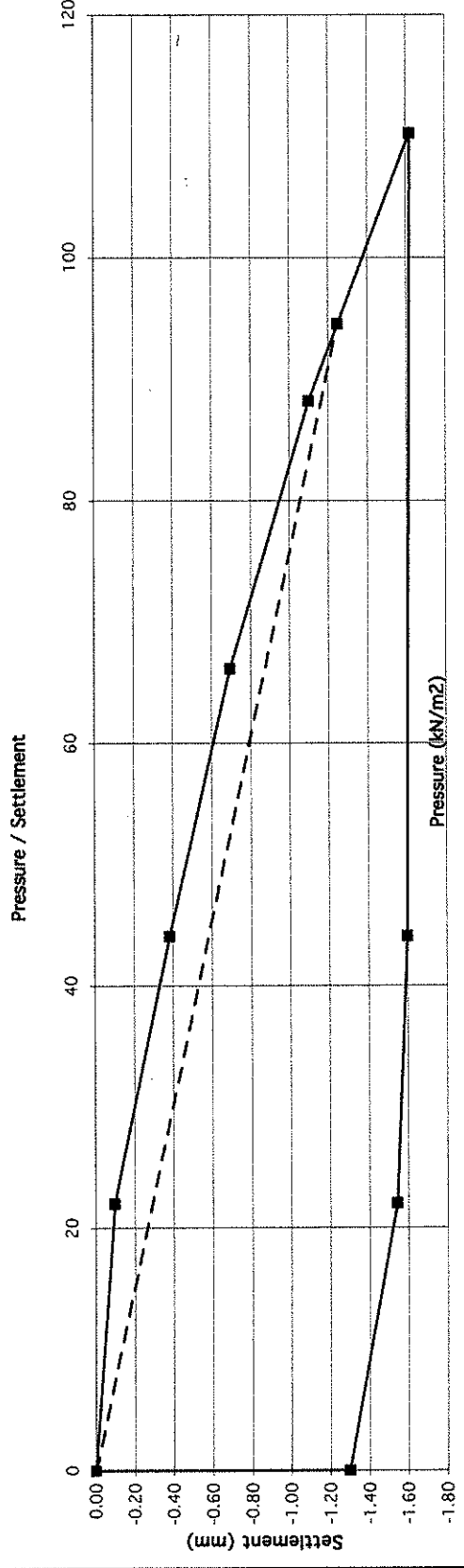
Modulus of subgrade reaction = 111 MPa/m

(Correction factor applied = 0.64 as per HD 25-26/10)

Equivalent CBR value in accordance with NRA HD25-26/10

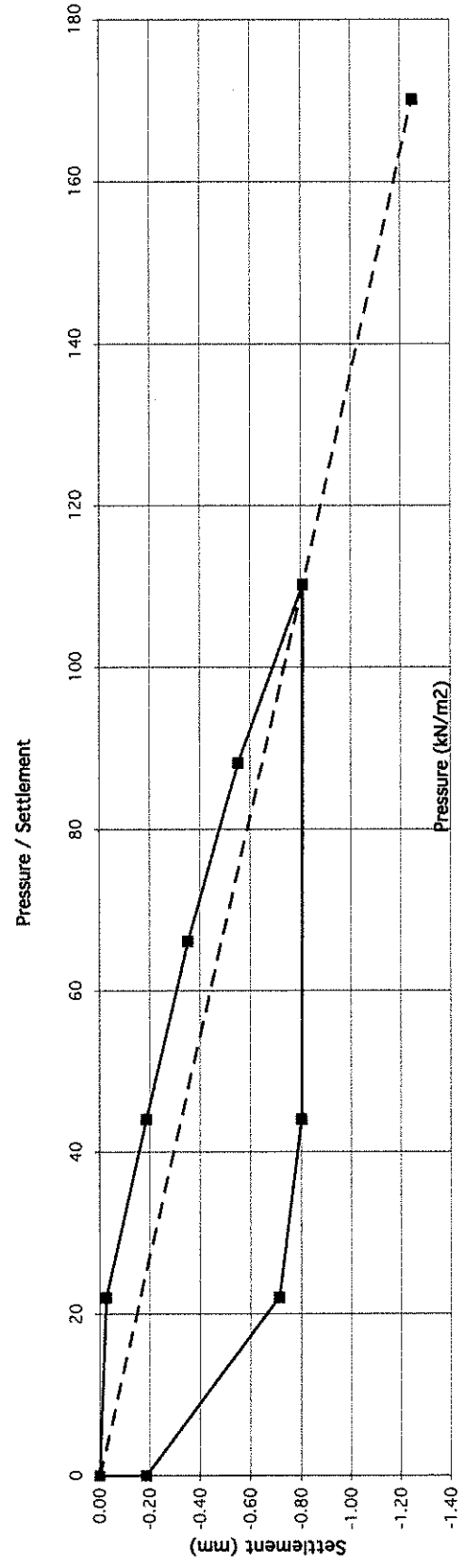
33.9 %

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No. R92264	Beech House, Scholarstown	Description of soil under test (natural soil, placed fill, sub-base) Light brown sandy SILT	 
Contract PBT 10 Load	Scholarstown, Co. Dublin		
Test No.	0.40mbgl	Easting (m)	
Location	DBFL Consulting Engineers	Northing (m)	
Depth	450 mm	Ground Level (mOD)	
Client	85 1377: Part 9: 1990 Test4 - Incremental Loading Test.	Sample Ref No.	N/A
Plate Diameter:	K. Kinsella	Depth	0.00 m bgl
Test Method			
Technician			
Authorised by			
Date	13/8/18		



Gradient at 1.25 mm settlement intersection = 76
 Modulus of subgrade reaction = 49 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10
 Equivalent CBR value in accordance with NRA HD25-26/10 **8.1 %**



PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92264	Description of soil under test (natural soil, placed fill, sub-base)	Light brown sandy SILT
Contract	Beech House, Scholarstown		
Test No.	PBT 10 ReLoad	Easting (m)	Northing (m)
Location	Scholarstown, Co. Dublin		
Depth	0.40mbgl	Ground Level (mOD)	
Client	DBFL Consulting Engineers	Sample Ref No.	N/A
Plate Diameter:	450 mm	Depth	0.00 m bgl
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test		
Technician	K. Kinsella		
Authorised by			
Date	08/13/2018		

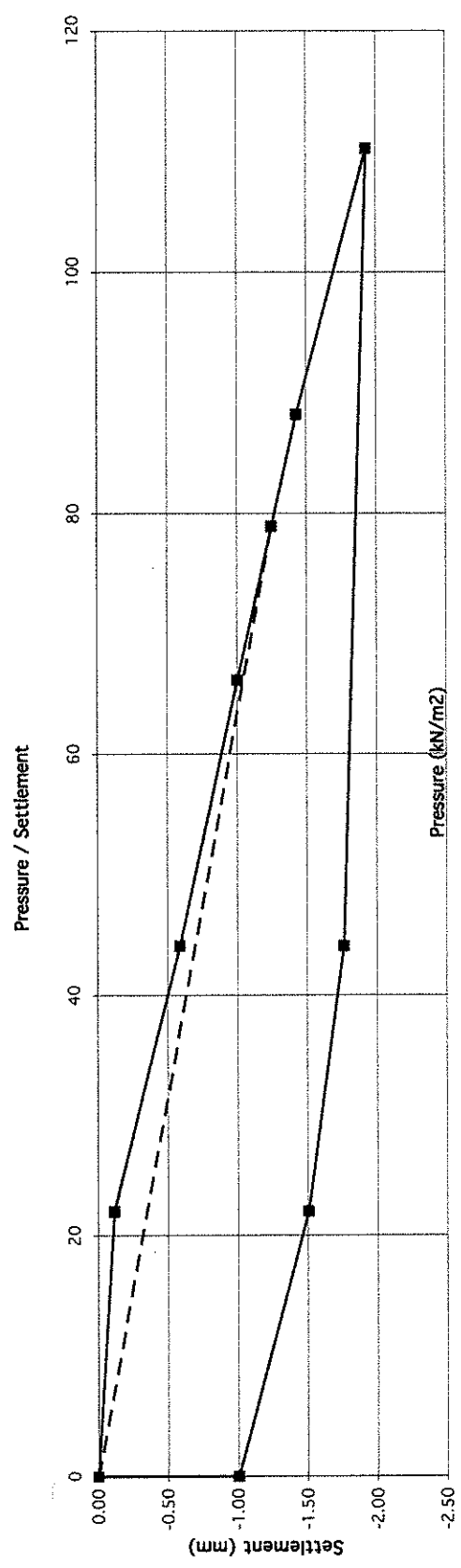


Gradient at 1.25 mm settlement intersection = 136
 Modulus of subgrade reaction = 88 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10

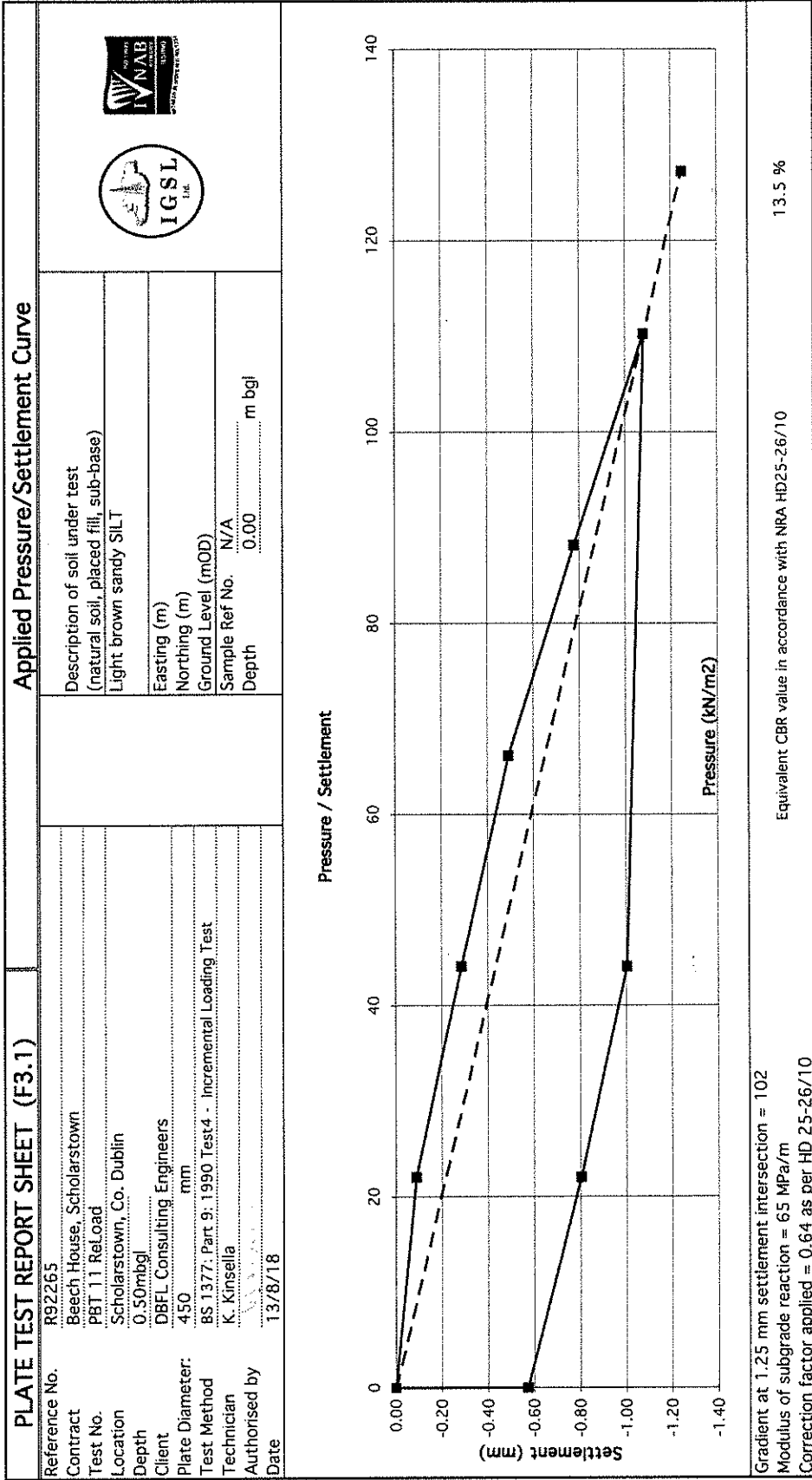
Equivalent CBR value in accordance with NRA HD25-26/10

22.4 %

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92265	Description of soil under test (natural soil, placed fill, sub-base)	 
Contract	Beech House, Scholarstown		
Test No.	PBT 11 Load	Light brown sandy SILT	Easting (m)
Location	Scholarstown, Co. Dublin		
Depth	0.50mbgl	Ground Level (mOD)	Sample Ref No.
Client	DBFL Consulting Engineers		
Plate Diameter:	450 mm	N/A	Depth
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test		
Technician	K. Kinsella	0.00 m bgl	
Authorised by			
Date	13/8/18		



Gradient at 1.25 mm settlement intersection = 63
 Modulus of subgrade reaction = 41 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10
 Equivalent CBR value in accordance with NRA HD25-26/10
 5.9 %



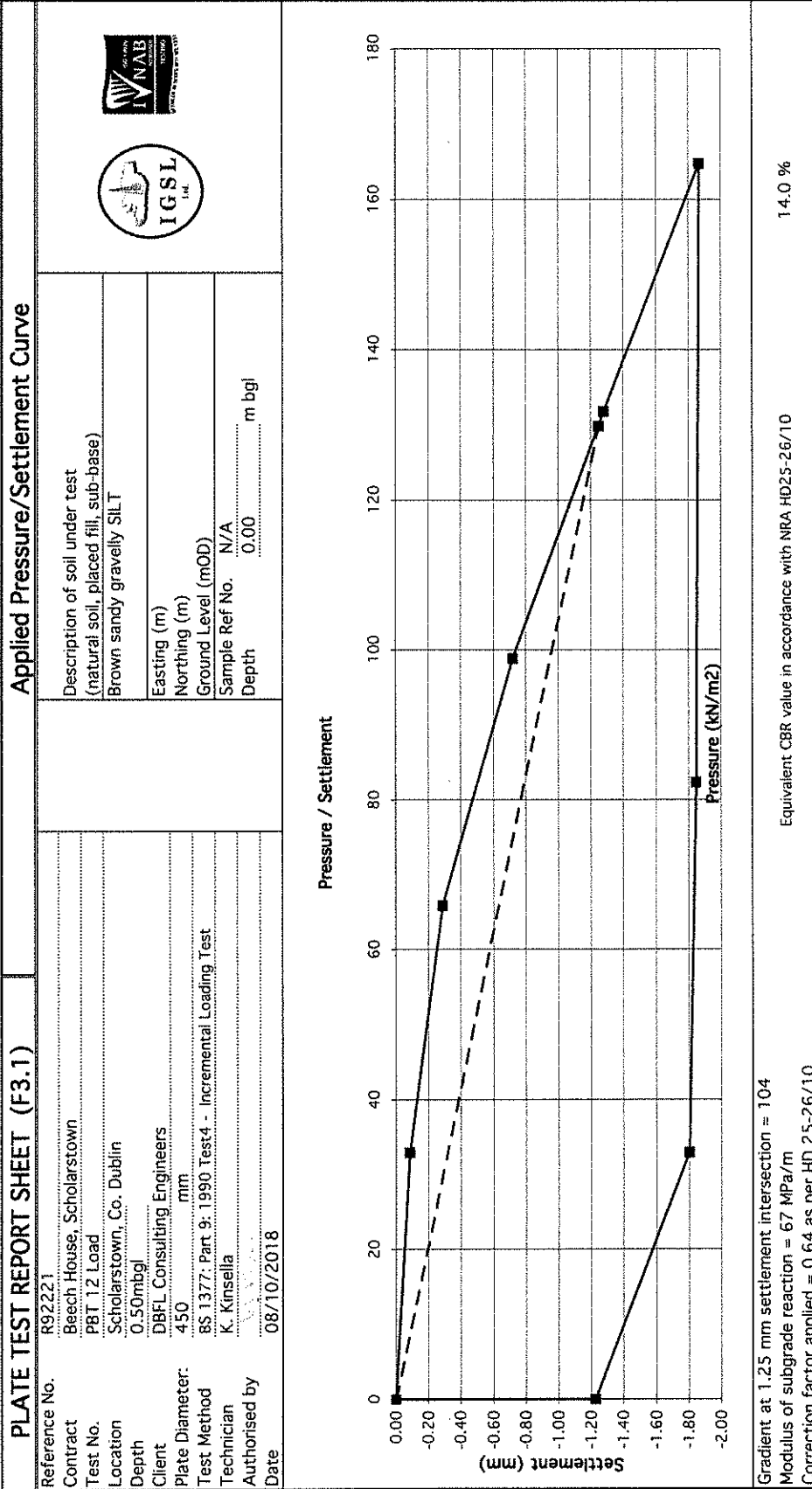
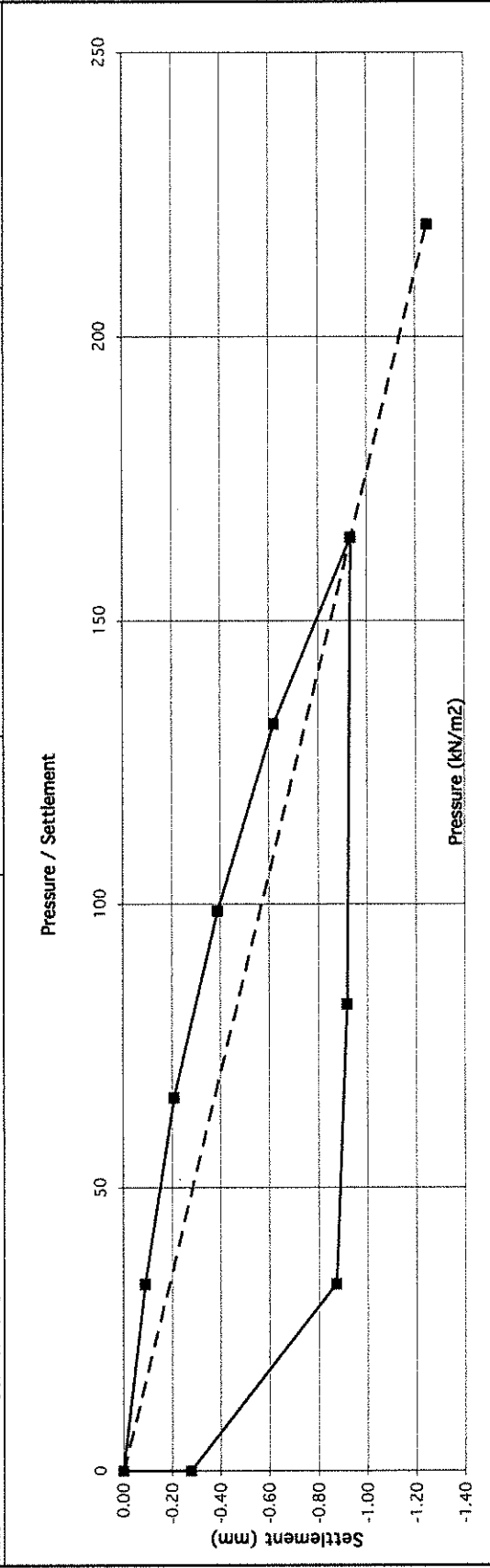




PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92221	Description of soil under test (natural soil, placed fill, sub-base)	IGSL Ireland Geotechnical Society Ltd.
Contract	Beech House, Scholarstown		
Test No.	PBT 12 ReLoad	Brown sandy gravelly SILT	IGSL Ireland Geotechnical Society Ltd.
Location	Scholarstown, Co. Dublin		
Depth	0.50mbgl	Easting (m)	
Client	DBFL Consulting Engineers	Northing (m)	
Plate Diameter:	450 mm	Ground Level (mOD)	
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	Sample Ref No.	N/A
Technician	K. Kinsella	Depth	0.00 m bgl
Authorised by			
Date	08/10/2018		

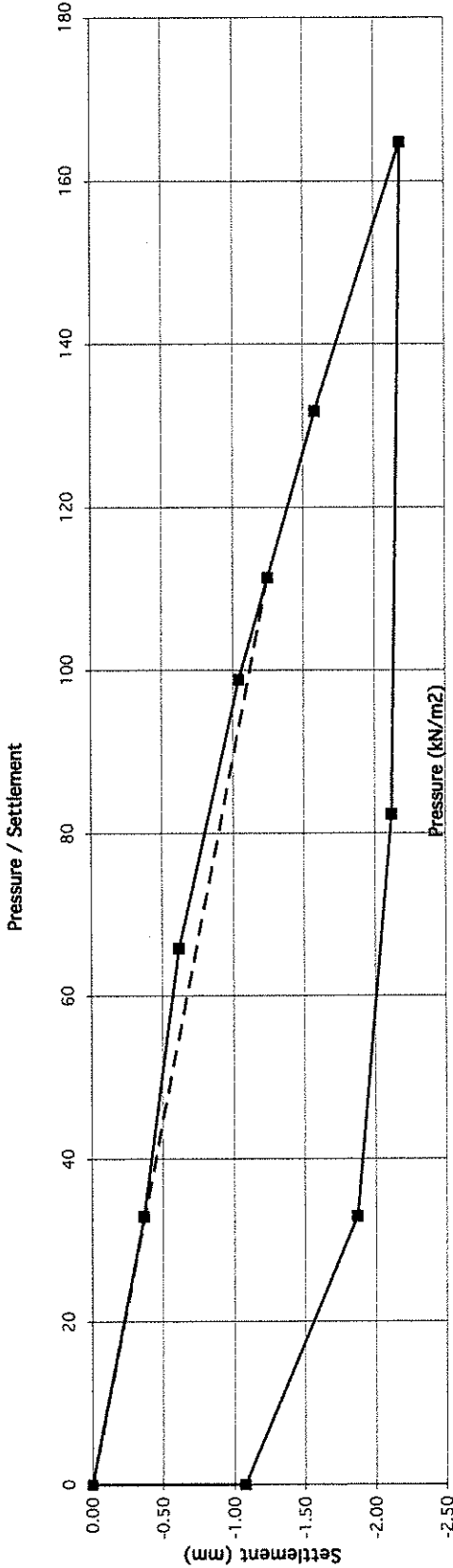


Gradient at 1.25 mm settlement intersection = 176
 Modulus of subgrade reaction = 113 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10

Equivalent CBR value in accordance with NRA HD25-26/10 **34.9 %**

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No. R92222	Beech House, Scholarstown	Description of soil under test (natural soil, placed fill, sub-base) Brown very sandy gravelly SILT	 
Contract PBT 13 Load	Scholarstown, Co. Dublin		
Location Scholarstown, Co. Dublin	Depth 0.50mbgl	Easting (m)	
Client DBFL Consulting Engineers	Plate Diameter: 450 mm	Northing (m)	
Test Method BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	Technician K. Kinsella	Ground Level (mOD)	
Authorised by	Date 08/10/2018	Sample Ref No.	N/A
		Depth	0.00 m bgl

Pressure / Settlement



Settlement (mm)

Pressure (kN/m²)

Equivalent CBR value in accordance with NRA HD25-26/10
10.7 %

Gradient at 1.25 mm settlement intersection = 89

Modulus of subgrade reaction = 57 MPa/m

Correction factor applied = 0.64 as per HD 25-26/10

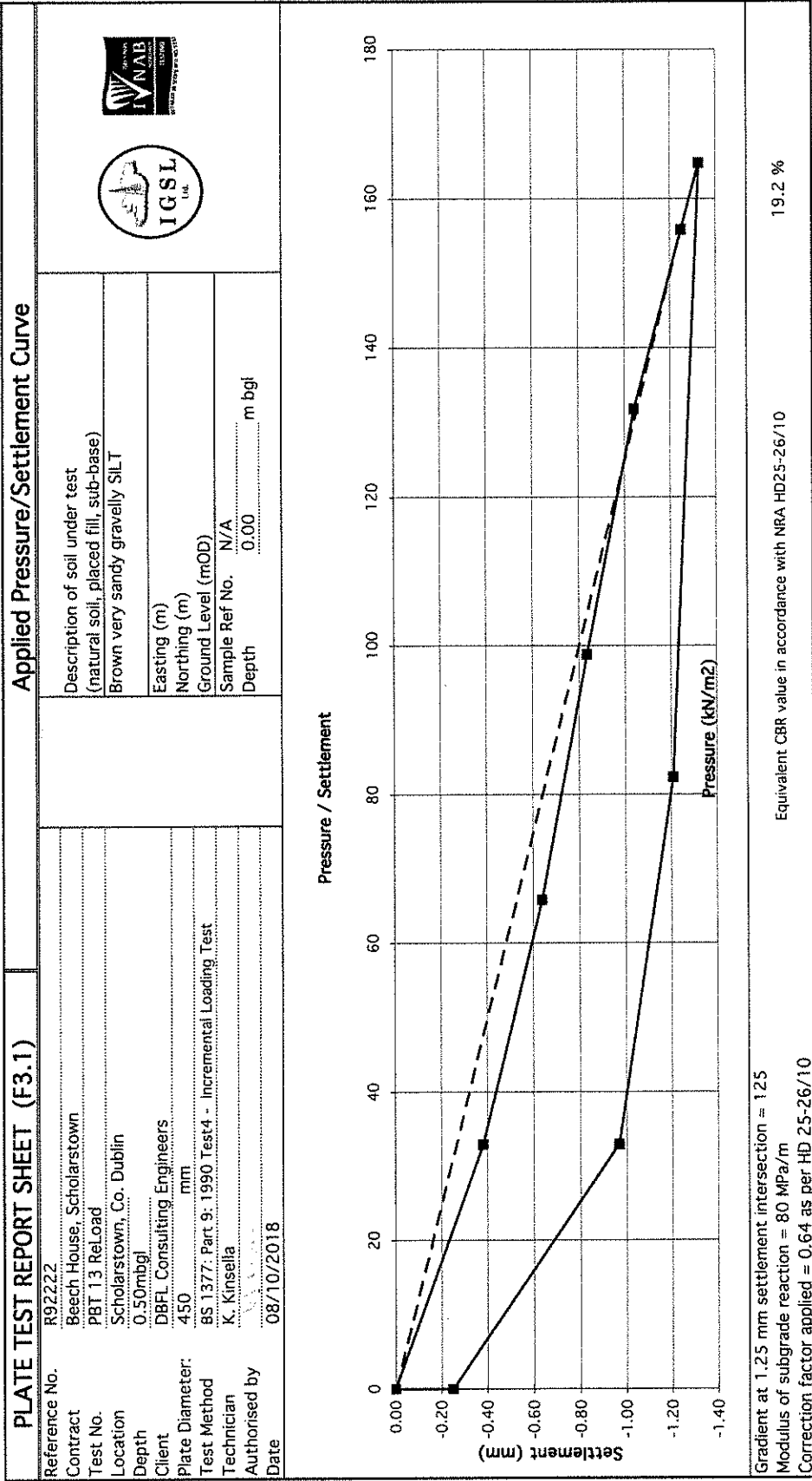


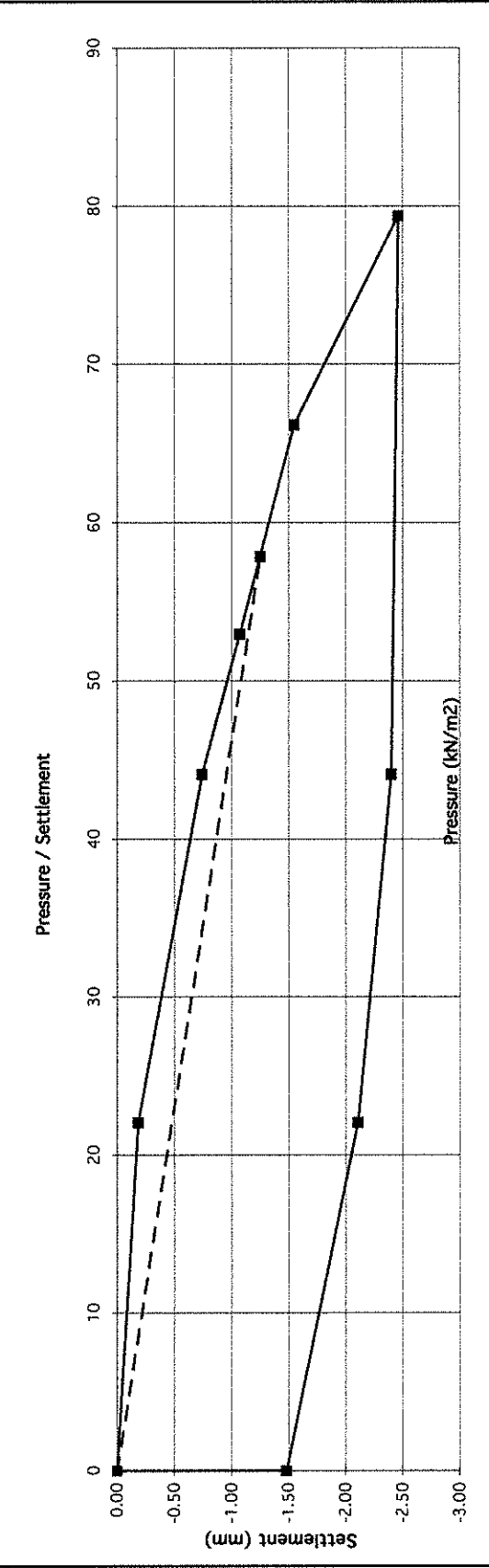
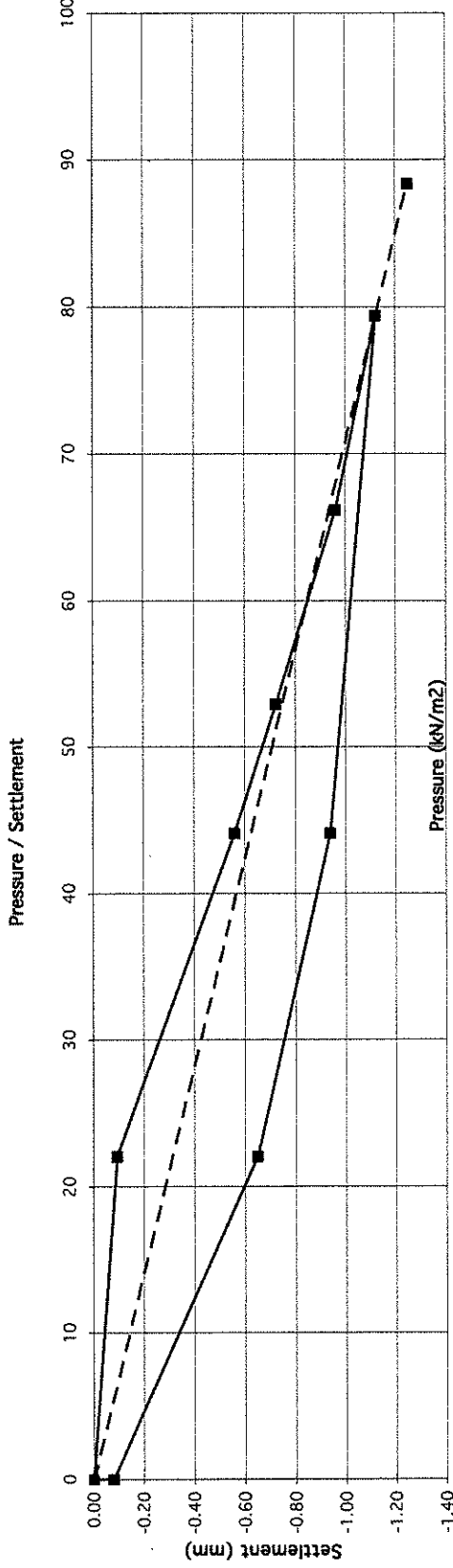


PLATE TEST REPORT SHEET (F3.1)	Applied Pressure/Settlement Curve
Reference No. R92266 Contract Beech House, Scholarstown Test No. PBT 14 Load Location Scholarstown, Co. Dublin Depth 0.40mbgl Client DBFL Consulting Engineers Plate Diameter: 450 mm Test Method BS 1377: Part 9: 1990 Test4 - Incremental Loading Test Technician K. Kinsella Authorised by Date 13/8/18	Description of soil under test (natural soil, placed fill, sub-base) Light brown sandy SILT with rare gravel Easting (m) Northing (m) Ground Level (mOD) Sample Ref No. N/A Depth 0.00 m bgl
 	





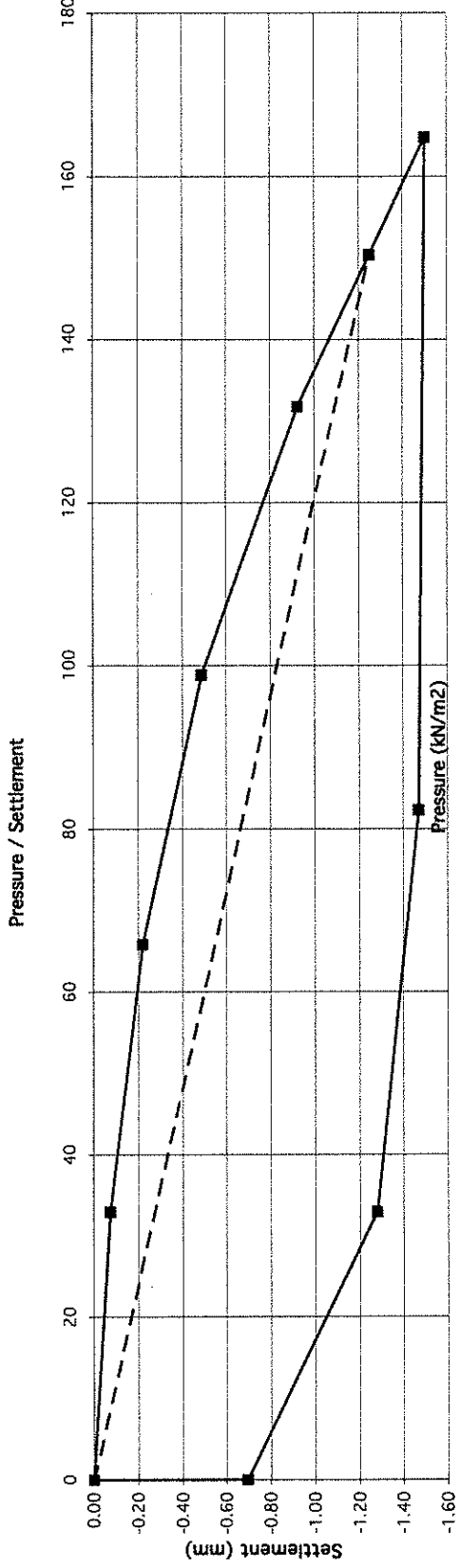
Gradient at 1.25 mm settlement intersection = 46
 Modulus of subgrade reaction = 30 MPa/m
 Correction factor applied = 0.64 as per HD 2.5-26/10
 Equivalent CBR value in accordance with NRA HD25-26/10
 3.5 %

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No.	R92266	Description of soil under test (natural soil, placed fill, sub-base)	IGSL INSTRUMENTED NATURAL ANALYSIS BENCHMARK
Contract	Beech House, Scholarstown		
Test No.	PBT 14 ReLoad	Light brown sandy SILT with rare gravel	IGSL INSTRUMENTED NATURAL ANALYSIS BENCHMARK
Location	Scholarstown, Co. Dublin		
Depth	0.40mbgl	Easting (m)	
Client	DBFL Consulting Engineers	Northing (m)	
Plate Diameter:	450 mm	Ground Level (mOD)	
Test Method	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	Sample Ref No.	N/A
Technician	K. Kinsealla	Depth	0.00 m bgl
Authorised by			
Date	13/8/18		



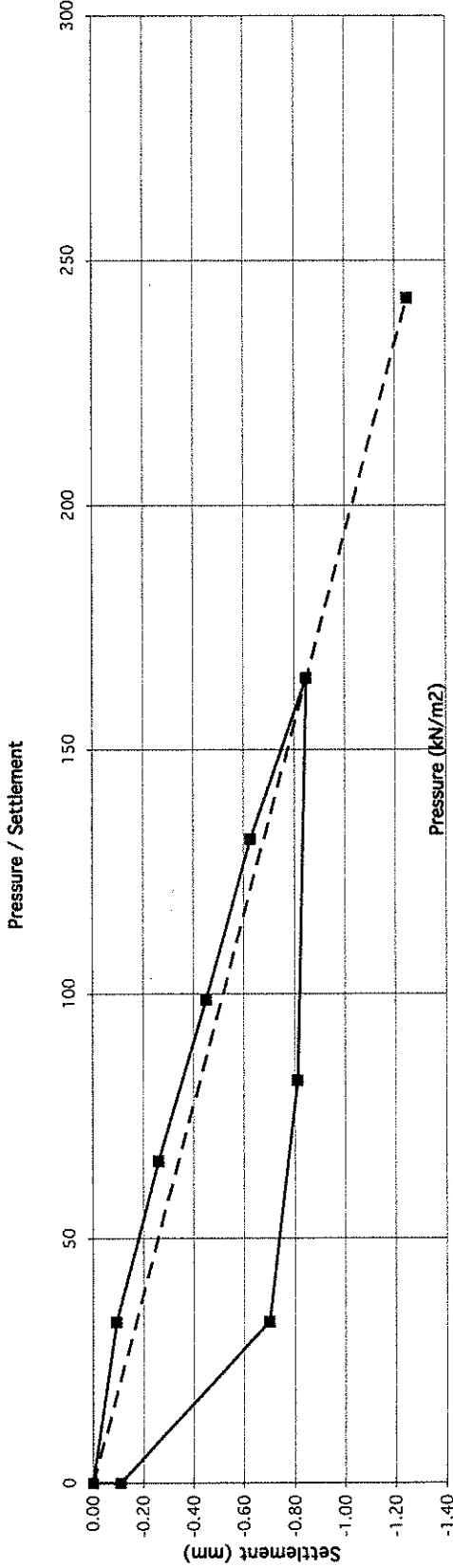


Gradient at 1.25 mm settlement intersection = 71
 Modulus of subgrade reaction = 45 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10
 Equivalent CBR value in accordance with NRA HD25-26/10
 7.2 %

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No. R92223	Beech House, Scholarstown	Description of soil under test (natural soil, placed fill, sub-base) Brown sandy slightly gravelly SILT	 
Contract PBT 15 Load	Scholarstown, Co. Dublin		
Test No.	0.50mbgl	Easting (m)	
Location	DBFL Consulting Engineers	Northing (m)	
Depth	450 mm	Ground Level (mOD)	
Client	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	Sample Ref No.	N/A
Plate Diameter:	K. Kinsella	Depth	0.00 m bgl
Test Method			
Technician			
Authorised by			
Date	08/10/2018		



Gradient at 1.25 mm settlement intersection = 120
 Modulus of subgrade reaction = 77 MPa/m
 Correction factor applied = 0.64 as per HD 25-26/10
 Equivalent CBR value in accordance with NRA HD25-26/10
 18.1 %

PLATE TEST REPORT SHEET (F3.1)		Applied Pressure/Settlement Curve	
Reference No. R92223	Beech House, Scholarstown	Description of soil under test (natural soil, placed fill, sub-base) Brown sandy slightly gravelly SILT	 
Contract PBT 15 ReLoad	Scholarstown, Co. Dublin		
Test No. 0.50mbgl	DBFL Consulting Engineers	Easting (m)	
Location Scholarstown, Co. Dublin	450 mm	Northing (m)	
Depth 0.50mbgl	BS 1377: Part 9: 1990 Test4 - Incremental Loading Test	Ground Level (mOD)	
Client DBFL Consulting Engineers	K. Kinsella	Sample Ref No.	N/A
Plate Diameter: 450 mm		Depth	0.00 m bgl
Test Method Incremental Loading Test			
Technician K. Kinsella			
Authorised by			
Date 08/10/2018			
			
Gradient at 1.25 mm settlement intersection = 194 Modulus of subgrade reaction = 125 MPa/m Correction factor applied = 0.64 as per HD 25-26/10		Equivalent CBR value in accordance with NRA HD25-26/10 41.3 %	

Appendix IV Dynamic Probes



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP01	
			SHEET Sheet 1 of 2	
CO-ORDINATES		DATE DRILLED 09/08/2018		
GROUND LEVEL (mOD)		DATE LOGGED 09/08/2018		
CLIENT		PROBE TYPE DPH		
ENGINEER DBFL Consulting Engineers		HAMMER MASS (kg) 50	INCREMENT SIZE (mm) 100	FALL HEIGHT (mm) 500

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	13	
0.10						0.10	19	
0.20						0.20	21	
0.30						0.30	25	
0.40						0.40	3	
0.50						0.50	12	
0.60						0.60	19	
0.70						0.70	18	
0.80						0.80	18	
0.90						0.90	14	
1.00						1.00	17	
1.10						1.10	10	
1.20						1.20	6	
1.30						1.30	6	
1.40						1.40	11	
1.50						1.50	11	
1.60						1.60	8	
1.70						1.70	8	
1.80						1.80	7	
1.90						1.90	8	
2.00						2.00	7	
2.10						2.10	11	
2.20						2.20	9	
2.30						2.30	7	
2.40						2.40	8	
2.50						2.50	7	
2.60						2.60	7	
2.70						2.70	7	
2.80						2.80	7	
2.90						2.90	8	
3.00						3.00	10	
3.10						3.10	8	
3.20						3.20	7	
3.30						3.30	6	
3.40						3.40	7	
3.50						3.50	8	
3.60						3.60	9	
3.70						3.70	8	
3.80						3.80	8	
3.90						3.90	7	
4.00						4.00	7	
4.10						4.10	8	
4.20						4.20	8	
4.30						4.30	7	
4.40						4.40	7	
4.50						4.50	10	
4.60						4.60	9	
4.70						4.70	10	
4.80						4.80	8	
4.90						4.90	8	

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL_GDT_14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown		PROBE NO. DP01
CO-ORDINATES		SHEET Sheet 2 of 2
GROUND LEVEL (mOD)	HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018
CLIENT	INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018
ENGINEER DBFL Consulting Engineers	FALL HEIGHT (mm) 500	PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record				
								0	5	10	15	20
5.0	(continued)					5.00	7					
						5.10	7					
						5.20	6					
						5.30	8					
						5.40	8					
						5.50	7					
						5.60	9					
						5.70	9					
						5.80	8					
						5.90	7					
6.0						6.00	6					
						6.10	5					
						6.20	6					
						6.30	6					
						6.40	6					
						6.50	6					
						6.60	7					
						6.70	8					
						6.80	7					
						6.90	6					
7.0						7.00	7					
						7.10	10					
						7.20	8					
						7.30	9					
						7.40	9					
						7.50	10					
						7.60	10					
						7.70	11					
						7.80	12					
8.0	End of Probe at 8.00 m					7.90	13					

GROUNDWATER OBSERVATIONS

REMARKS

JGSL DP LOG 100MM INCREMENTS 21167.GPJ JGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown		PROBE NO. DP02
CO-ORDINATES		SHEET Sheet 1 of 1
GROUND LEVEL (mOD)	HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018
CLIENT	INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018
ENGINEER DBFL Consulting Engineers	FALL HEIGHT (mm) 500	PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record	
0.0	End of Probe at 1.50 m					0.00	6		
0.10							0.10		12
0.20							0.20		17
0.30							0.30		16
0.40							0.40		14
0.50							0.50		11
0.60							0.60		10
0.70							0.70		11
0.80							0.80		14
0.90							0.90		15
1.00							1.00		17
1.10							1.10		20
1.20							1.20		23
1.30							1.30		27
1.40							1.40		25

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP03
CO-ORDINATES			SHEET Sheet 1 of 1
GROUND LEVEL (mOD)	HAMMER MASS (kg) 50		DATE DRILLED 10/08/2018
CLIENT	INCREMENT SIZE (mm) 100		DATE LOGGED 10/08/2018
ENGINEER DBFL Consulting Engineers	FALL HEIGHT (mm) 500		PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
						0.10	11	
						0.20	16	
						0.30	17	
						0.40	16	
						0.50	13	
						0.60	9	
						0.70	6	
						0.80	7	
						0.90	7	
1.0						1.00	8	
						1.10	9	
						1.20	9	
						1.30	7	
						1.40	7	
						1.50	6	
						1.60	8	
						1.70	14	
						1.80	24	
						1.90	25	
2.0	End of Probe at 2.00 m							
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS: 21167.GPJ IGSL_GDT_14/08/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP04	
CO-ORDINATES			SHEET Sheet 1 of 1	
GROUND LEVEL (mOD)		HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018	
CLIENT		INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm) 500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	5	
						0.10	16	
						0.20	18	
						0.30	17	
						0.40	15	
						0.50	11	
						0.60	8	
						0.70	9	
						0.80	17	
						0.90	12	
1.0						1.00	14	
						1.10	20	
						1.20	18	
						1.30	15	
						1.40	17	
						1.50	18	
						1.60	20	
						1.70	31	
						1.80	25	
2.0	End of Probe at 1.90 m							
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown				PROBE NO. DP05	
CO-ORDINATES				SHEET Sheet 1 of 1	
GROUND LEVEL (mOD)		HAMMER MASS (kg)	50	DATE DRILLED 09/08/2018	
CLIENT		INCREMENT SIZE (mm)	100	DATE LOGGED 09/08/2018	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm)	500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	7	
						0.10	15	
						0.20	23	
						0.30	24	
						0.40	21	
						0.50	20	
						0.60	23	
						0.70	27	
						0.80	25	
1.0	End of Probe at 0.90 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSSL DP LOG 100MM INCREMENTS 21167.GPJ IGSLGDT 14/08/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP06
CO-ORDINATES			SHEET Sheet 1 of 1
GROUND LEVEL (mOD)	HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018	
CLIENT	INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018	
ENGINEER DBFL Consulting Engineers	FALL HEIGHT (mm) 500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	7	
0.10						0.10	17	
0.20						0.20	24	
0.30						0.30	25	
0.40						0.40	20	
0.50						0.50	16	
0.60						0.60	11	
0.70						0.70	5	
0.80						0.80	4	
0.90						0.90	4	
1.00						1.00	3	
1.10						1.10	3	
1.20						1.20	2	
1.30						1.30	2	
1.40						1.40	14	
1.50						1.50	16	
1.60						1.60	19	
1.70						1.70	12	
1.80						1.80	6	
1.90						1.90	4	
2.00						2.00	3	
2.10						2.10	3	
2.20						2.20	2	
2.30						2.30	9	
2.40						2.40	5	
2.50						2.50	5	
2.60						2.60	7	
2.70						2.70	7	
2.80						2.80	25	
2.90						2.90	22	
3.00						3.00	23	
3.10						3.10	25	
	End of Probe at 3.20 m							

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOC 100MM INCREMENTS 21167.GPJ IGSL_GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown		PROBE NO. DP07
CO-ORDINATES		SHEET Sheet 1 of 1
GROUND LEVEL (mOD)	HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018
CLIENT	INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018
ENGINEER DBFL Consulting Engineers	FALL HEIGHT (mm) 500	PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	5	
0.10						0.10	9	
0.20						0.20	12	
0.30						0.30	17	
0.40						0.40	15	
0.50						0.50	16	
0.60						0.60	16	
0.70						0.70	14	
0.80						0.80	19	
0.90						0.90	29	
1.00						1.00	25	
1.0	End of Probe at 1.10 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.CPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP08	
CO-ORDINATES			SHEET Sheet 1 of 1	
GROUND LEVEL (mOD)		HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018	
CLIENT		INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm) 500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
						0.10	9	
						0.20	13	
						0.30	19	
						0.40	16	
						0.50	18	
						0.60	19	
						0.70	14	
						0.80	13	
						0.90	3	
						1.00	2	
						1.10	3	
						1.20	3	
						1.30	4	
						1.40	6	
						1.50	7	
						1.60	7	
						1.70	8	
						1.80	8	
						1.90	15	
						2.00	18	
						2.10	19	
						2.20	25	
	End of Probe at 2.30 m							

GROUNDWATER OBSERVATIONS

REMARKS

IGSSL DP LOG 100MM INCREMENTS 21167.GPJ IGSSL_GDT_14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP09	
CO-ORDINATES			SHEET Sheet 1 of 1	
GROUND LEVEL (mOD)		HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018	
		INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018	
CLIENT		FALL HEIGHT (mm) 500	PROBE TYPE DPH	
ENGINEER DBFL Consulting Engineers				

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	5	
						0.10	9	
						0.20	16	
						0.30	18	
						0.40	15	
						0.50	17	
						0.60	15	
						0.70	14	
						0.80	9	
						0.90	9	
1.0						1.00	9	
						1.10	10	
						1.20	11	
						1.30	7	
						1.40	14	
						1.50	22	
						1.60	19	
						1.70	20	
						1.80	22	
2.0	End of Probe at 2.10 m					1.90	23	
						2.00	25	

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown				PROBE NO. DP10	
CO-ORDINATES				SHEET Sheet 1 of 1	
GROUND LEVEL (mOD)		HAMMER MASS (kg)	50	DATE DRILLED 09/08/2018	
CLIENT		INCREMENT SIZE (mm)	100	DATE LOGGED 09/08/2018	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm)	500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	5	
						0.10	7	
						0.20	8	
						0.30	11	
						0.40	12	
						0.50	22	
						0.60	19	
						0.70	11	
						0.80	7	
						0.90	10	
1.0						1.00	16	
						1.10	13	
						1.20	10	
						1.30	10	
						1.40	13	
						1.50	16	
						1.60	23	
						1.70	29	
						1.80	25	
2.0	End of Probe at 1.90 m							
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL_GDT 14/6/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP11	
CO-ORDINATES			SHEET Sheet 1 of 1	
GROUND LEVEL (mOD)		HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018	
CLIENT		INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm) 500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
						0.10	12	
						0.20	16	
						0.30	15	
						0.40	14	
						0.50	15	
						0.60	11	
						0.70	13	
						0.80	14	
						0.90	15	
						1.00	13	
						1.10	15	
						1.20	18	
						1.30	19	
						1.40	21	
						1.50	27	
						1.60	25	
1.0								
2.0	End of Probe at 1.70 m							
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS, 21167.GPJ IGSL_GDT_14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP12	
CO-ORDINATES			SHEET Sheet 1 of 1	
GROUND LEVEL (mOD)		HAMMER MASS (kg) 50	DATE DRILLED 10/08/2018	
CLIENT		INCREMENT SIZE (mm) 100	DATE LOGGED 10/08/2018	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm) 500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	4	
						0.10	10	
						0.20	12	
						0.30	15	
						0.40	14	
						0.50	10	
						0.60	8	
						0.70	7	
						0.80	6	
						0.90	7	
1.0						1.00	7	
						1.10	9	
						1.20	10	
						1.30	8	
						1.40	7	
						1.50	12	
						1.60	19	
						1.70	24	
						1.80	25	
2.0	End of Probe at 1.90 m							
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSSL DP LOG 100MM INCREMENTS 21167.GPJ IGSSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP13	
			SHEET Sheet 1 of 1	
CO-ORDINATES		DATE DRILLED 09/08/2018		
GROUND LEVEL (mOD)		DATE LOGGED 09/08/2018		
CLIENT		PROBE TYPE DPH		
ENGINEER DBFL Consulting Engineers		HAMMER MASS (kg) 50	INCREMENT SIZE (mm) 100	FALL HEIGHT (mm) 500

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
						0.10	10	
						0.20	12	
						0.30	10	
						0.40	10	
						0.50	8	
						0.60	12	
						0.70	21	
						0.80	20	
						0.90	14	
						1.00	21	
						1.10	25	
1.0	End of Probe at 1.20 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL_DP_LOG_100MM_INCREMENT_21167.GPJ IGSL_GDT_14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown

PROBE NO. **DP14**

CO-ORDINATES

SHEET Sheet 1 of 1

GROUND LEVEL (mOD)

HAMMER MASS (kg) 50

DATE DRILLED 10/08/2018

DATE LOGGED 10/08/2018

CLIENT
ENGINEER DBFL Consulting Engineers

INCREMENT SIZE (mm) 100

FALL HEIGHT (mm) 500

PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
0.10						0.10	9	
0.20						0.20	14	
0.30						0.30	12	
0.40						0.40	9	
0.50						0.50	9	
0.60						0.60	7	
0.70						0.70	5	
0.80						0.80	5	
0.90						0.90	7	
1.00						1.00	15	
1.10						1.10	12	
1.20						1.20	5	
1.30						1.30	8	
1.40						1.40	8	
1.50						1.50	18	
1.60						1.60	21	
1.70						1.70	23	
1.80						1.80	25	
2.0	End of Probe at 1.90 m							
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP15
CO-ORDINATES			SHEET Sheet 1 of 1
GROUND LEVEL (mOD)	HAMMER MASS (kg)	50	DATE DRILLED 10/08/2018
CLIENT	INCREMENT SIZE (mm)	100	DATE LOGGED 10/08/2018
ENGINEER DBFL Consulting Engineers	FALL HEIGHT (mm)	500	PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0	End of Probe at 1.60 m					0.00	6	
0.10						0.10	15	
0.20						0.20	19	
0.30						0.30	29	
0.40						0.40	17	
0.50						0.50	7	
0.60						0.60	10	
0.70						0.70	12	
0.80						0.80	13	
0.90						0.90	18	
1.00						1.00	15	
1.10						1.10	14	
1.20						1.20	12	
1.30						1.30	25	
1.40						1.40	28	
1.50					1.50	25		

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/6/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown

PROBE NO. DP16

CO-ORDINATES

SHEET Sheet 1 of 1

GROUND LEVEL (mOD)

HAMMER MASS (kg) 50

DATE DRILLED 10/08/2018

DATE LOGGED 10/08/2018

CLIENT

INCREMENT SIZE (mm) 100

ENGINEER DBFL Consulting Engineers

FALL HEIGHT (mm) 500

PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
						0.10	8	
						0.20	12	
						0.30	12	
						0.40	12	
						0.50	10	
						0.60	10	
						0.70	10	
						0.80	12	
						0.90	14	
1.0						1.00	21	
						1.10	19	
						1.20	25	
						1.30	32	
						1.40	25	
	End of Probe at 1.50 m							

GROUNDWATER OBSERVATIONS

REMARKS

IGSL.DP.LOG.100MM.INCREMENTS.21167.GPJ IGSL.GDT.14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP17	
			SHEET Sheet 1 of 1	
CO-ORDINATES		DATE DRILLED 10/08/2018		
GROUND LEVEL (mOD)		HAMMER MASS (kg) 50	DATE LOGGED 10/08/2018	
CLIENT		INCREMENT SIZE (mm) 100	PROBE TYPE DPH	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm) 500		

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	4	
0.10						0.10	10	
0.20						0.20	11	
0.30						0.30	15	
0.40						0.40	17	
0.50						0.50	12	
0.60						0.60	8	
0.70						0.70	6	
0.80						0.80	8	
0.90						0.90	12	
1.00						1.00	26	
1.10						1.10	26	
1.20						1.20	16	
1.30						1.30	14	
1.40						1.40	20	
1.50						1.50	25	
	End of Probe at 1.60 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL_DP_LOG_100MM_INCREMENT_21167.GPJ IGSL_GDT_14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP18	
			SHEET Sheet 1 of 1	
CO-ORDINATES		DATE DRILLED 09/08/2018		
GROUND LEVEL (mOD)		DATE LOGGED 09/08/2018		
CLIENT		PROBE TYPE DPH		
ENGINEER DBFL Consulting Engineers		HAMMER MASS (kg) 50	INCREMENT SIZE (mm) 100	FALL HEIGHT (mm) 500

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	2	
						0.10	9	
						0.20	14	
						0.30	14	
						0.40	11	
						0.50	12	
						0.60	13	
						0.70	27	
						0.80	32	
						0.90	25	
1.0	End of Probe at 1.00 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown

PROBE NO. DP19

SHEET Sheet 1 of 1

CO-ORDINATES

GROUND LEVEL (mOD)

HAMMER MASS (kg) 50

DATE DRILLED 10/08/2018

DATE LOGGED 10/08/2018

CLIENT

INCREMENT SIZE (mm) 100

PROBE TYPE DPH

ENGINEER DBFL Consulting Engineers

FALL HEIGHT (mm) 500

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	8	
0.10						0.10	23	
0.20						0.20	34	
0.30						0.30	23	
0.40						0.40	27	
0.50						0.50	23	
0.60						0.60	29	
0.70						0.70	38	
0.80						0.80	25	
1.0	End of Probe at 0.90 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL_DP_LOG_100MM_INCREMENTALS_21167.GPJ IGSL_GDT_14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP20
			SHEET Sheet 1 of 1
CO-ORDINATES		HAMMER MASS (kg) 50	DATE DRILLED 10/08/2018
GROUND LEVEL (mOD)		INCREMENT SIZE (mm) 100	DATE LOGGED 10/08/2018
CLIENT ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm) 500	PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record	
0.0	End of Probe at 1.60 m					0.00	4		
0.10							0.10		8
0.20							0.20		14
0.30							0.30		14
0.40							0.40		15
0.50							0.50		11
0.60							0.60		8
0.70							0.70		6
0.80							0.80		5
0.90							0.90		3
1.00							1.00		13
1.10							1.10		10
1.20							1.20		11
1.30							1.30		14
1.40							1.40		22
1.50							1.50		25

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP21
CO-ORDINATES			SHEET Sheet 1 of 1
GROUND LEVEL (mOD)	HAMMER MASS (kg) 50	DATE DRILLED 10/08/2018	
CLIENT	INCREMENT SIZE (mm) 100	DATE LOGGED 10/08/2018	
ENGINEER DBFL Consulting Engineers	FALL HEIGHT (mm) 500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
						0.10	9	
						0.20	13	
						0.30	14	
						0.40	10	
						0.50	10	
						0.60	7	
						0.70	8	
						0.80	12	
						0.90	9	
1.0						1.00	15	
						1.10	17	
						1.20	22	
						1.30	15	
						1.40	18	
						1.50	16	
						1.60	24	
						1.70	18	
						1.80	19	
						1.90	23	
2.0						2.00	24	
	End of Probe at 2.20 m					2.10	25	

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS, 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown				PROBE NO. DP22	
				SHEET Sheet 1 of 1	
CO-ORDINATES		HAMMER MASS (kg) 50		DATE DRILLED 10/08/2018	
GROUND LEVEL (mOD)		INCREMENT SIZE (mm) 100		DATE LOGGED 10/08/2018	
CLIENT		FALL HEIGHT (mm) 500		PROBE TYPE DPH	
ENGINEER DBFL Consulting Engineers					

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	7	
						0.10	19	
						0.20	21	
						0.30	20	
						0.40	17	
						0.50	19	
						0.60	17	
						0.70	19	
						0.80	26	
						0.90	25	
1.0	End of Probe at 1.00 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL_DP_LOG_100MMI_INCREMENTS_21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP23	
			SHEET Sheet 1 of 1	
CO-ORDINATES		DATE DRILLED 10/08/2018		
GROUND LEVEL (mOD)		HAMMER MASS (kg) 50	DATE LOGGED 10/08/2018	
CLIENT		INCREMENT SIZE (mm) 100	PROBE TYPE DPH	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm) 500		

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
0.10						0.10	10	
0.20						0.20	15	
0.30						0.30	9	
0.40						0.40	9	
0.50						0.50	7	
0.60						0.60	9	
0.70						0.70	8	
0.80						0.80	5	
0.90						0.90	3	
1.00						1.00	2	
1.10						1.10	2	
1.20						1.20	3	
1.30						1.30	4	
1.40						1.40	3	
1.50						1.50	3	
1.60						1.60	2	
1.70						1.70	1	
1.80						1.80	1	
1.90						1.90	2	
2.00						2.00	2	
2.10						2.10	3	
2.20						2.20	5	
2.30						2.30	11	
2.40						2.40	13	
2.50						2.50	20	
2.60						2.60	21	
2.70						2.70	22	
2.80						2.80	21	
2.90						2.90	24	
3.00						3.00	25	
	End of Probe at 3.10 m							

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP24	
			SHEET Sheet 1 of 1	
CO-ORDINATES		DATE DRILLED 10/08/2018		
GROUND LEVEL (mOD)		DATE LOGGED 10/08/2018		
CLIENT		PROBE TYPE DPH		
ENGINEER DBFL Consulting Engineers		HAMMER MASS (kg) 50	INCREMENT SIZE (mm) 100	FALL HEIGHT (mm) 500

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	4	
0.10						0.10	8	
0.20						0.20	12	
0.30						0.30	10	
0.40						0.40	8	
0.50						0.50	7	
0.60						0.60	8	
0.70						0.70	8	
0.80						0.80	6	
0.90						0.90	4	
1.00						1.00	5	
1.10						1.10	8	
1.20						1.20	9	
1.30						1.30	10	
1.40						1.40	12	
1.50						1.50	18	
1.60						1.60	21	
1.70						1.70	25	
2.0	End of Probe at 1.80 m							
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown

PROBE NO. DP25

CO-ORDINATES

SHEET Sheet 1 of 1

GROUND LEVEL (mOD)

HAMMER MASS (kg) 50

DATE DRILLED 10/08/2018

CLIENT ENGINEER DBFL Consulting Engineers

INCREMENT SIZE (mm) 100

DATE LOGGED 10/08/2018

FALL HEIGHT (mm) 500

PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	2	
						0.10	12	
						0.20	16	
						0.30	16	
						0.40	14	
						0.50	12	
						0.60	20	
						0.70	38	
						0.80	25	
1.0	End of Probe at 0.90 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL_DP_LOG_100MM_INCREMENTS_21167.GPJ IGSL_GDT_14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP26	
			SHEET Sheet 1 of 1	
CO-ORDINATES		DATE DRILLED 10/08/2018		
GROUND LEVEL (mOD)		DATE LOGGED 10/08/2018		
CLIENT		PROBE TYPE DPH		
ENGINEER DBFL Consulting Engineers		HAMMER MASS (kg) 50	INCREMENT SIZE (mm) 100	FALL HEIGHT (mm) 500

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
						0.10	9	
						0.20	16	
						0.30	19	
						0.40	18	
						0.50	17	
						0.60	29	
						0.70	39	
						0.80	25	
1.0	End of Probe at 0.90 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS: 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholiarstown		PROBE NO. DP27
CO-ORDINATES		SHEET Sheet 1 of 1
GROUND LEVEL (mOD)	HAMMER MASS (kg) 50	DATE DRILLED 10/08/2018
CLIENT	INCREMENT SIZE (mm) 100	DATE LOGGED 10/08/2018
ENGINEER DBFL Consulting Engineers	FALL HEIGHT (mm) 500	PROBE TYPE DPH

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record	
0.0	End of Probe at 0.80 m					0.00	3		
						0.10	7		
						0.20	14		
						0.30	17		
						0.40	12		
						0.50	26		
						0.60	29		
						0.70	25		
1.0									
2.0									
3.0									
4.0									

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL_GDT_14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER
21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP28	
			SHEET Sheet 1 of 1	
CO-ORDINATES		DATE DRILLED 09/08/2018		
GROUND LEVEL (mOD)		DATE LOGGED 09/08/2018		
CLIENT		PROBE TYPE DPH		
ENGINEER DBFL Consulting Engineers		HAMMER MASS (kg) 50	INCREMENT SIZE (mm) 100	FALL HEIGHT (mm) 500

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	3	
						0.10	8	
						0.20	17	
						0.30	13	
						0.40	22	
						0.50	10	
						0.60	9	
						0.70	13	
						0.80	9	
						0.90	7	
1.0						1.00	29	
	End of Probe at 1.20 m					1.10	25	
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/8/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP29	
CO-ORDINATES			SHEET Sheet 1 of 1	
GROUND LEVEL (mOD)		HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018	
CLIENT		INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm) 500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	4	
						0.10	8	
						0.20	13	
						0.30	14	
						0.40	13	
						0.50	12	
						0.60	8	
						0.70	7	
						0.80	5	
						0.90	3	
						1.00	4	
						1.10	4	
						1.20	3	
						1.30	5	
						1.40	5	
						1.50	5	
						1.60	6	
						1.70	7	
						1.80	13	
						1.90	6	
						2.00	26	
						2.10	17	
						2.20	6	
						2.30	15	
						2.40	22	
						2.50	25	
						2.60	25	
	End of Probe at 2.70 m							

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL_GDT 14/6/18



DYNAMIC PROBE RECORD

REPORT NUMBER

21167

CONTRACT Beech House, Scholarstown			PROBE NO. DP30	
CO-ORDINATES			SHEET Sheet 1 of 1	
GROUND LEVEL (mOD)		HAMMER MASS (kg) 50	DATE DRILLED 09/08/2018	
CLIENT		INCREMENT SIZE (mm) 100	DATE LOGGED 09/08/2018	
ENGINEER DBFL Consulting Engineers		FALL HEIGHT (mm) 500	PROBE TYPE DPH	

Depth (m)	Geotechnical Description	Legend	Depth (m)	Elevation (mOD)	Water	Depth (m)	Probe Readings (Blows/Increment)	Graphic Probe Record
0.0						0.00	5	
0.10						0.10	9	
0.20						0.20	12	
0.30						0.30	13	
0.40						0.40	10	
0.50						0.50	10	
0.60						0.60	26	
0.70						0.70	19	
0.80						0.80	19	
0.90						0.90	14	
1.00						1.00	23	
1.10						1.10	24	
1.20						1.20	19	
1.30						1.30	23	
1.40						1.40	28	
1.50						1.50	25	
1.60	End of Probe at 1.60 m							
2.0								
3.0								
4.0								

GROUNDWATER OBSERVATIONS

REMARKS

IGSL DP LOG 100MM INCREMENTS 21167.GPJ IGSL.GDT 14/8/18

Appendix V BRE Digest 365 Tests

Soakaway Design f-value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT01 (1st cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712569.377
 726990.853
 79.908

Summary of ground conditions

from	to	Description	Ground water
0.00	0.20	Firm brown TOPSOIL with rootlets	No water
0.20	0.70	Stiff light brown sandy slightly gravelly SILT with occasional cobbles	
0.70	2.00	Stiff to firm brown sandy slightly gravelly cobbly SILT	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.310	0.00
1.310	1.00
1.310	2.00
1.310	3.00
1.310	4.00
1.310	5.00
1.310	6.00
1.310	7.00
1.310	8.00
1.310	9.00
1.310	10.00
1.310	12.00
1.310	14.00
1.310	16.00
1.310	18.00
1.310	20.00
1.310	25.00
1.320	30.00
1.320	40.00
1.320	50.00
1.320	60.00

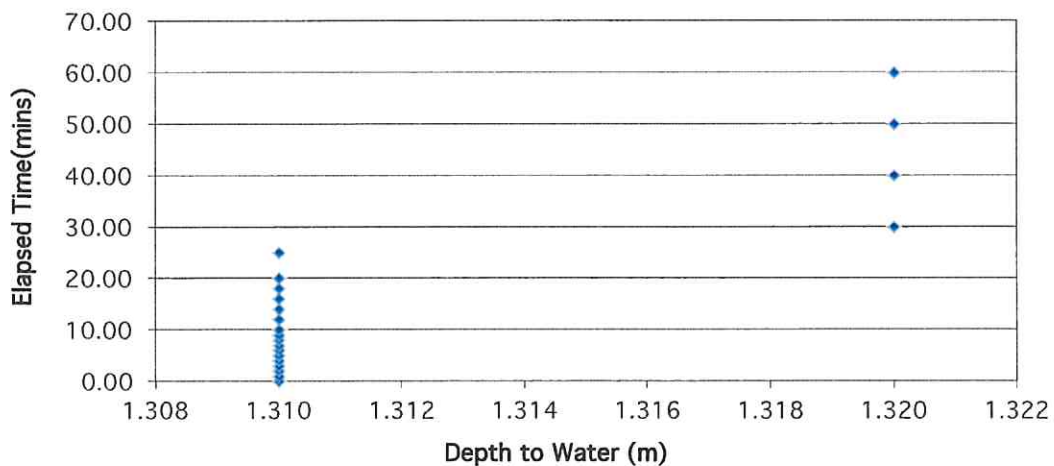
Field Test

Depth of Pit (D)	2.00	m
Width of Pit (B)	0.60	m
Length of Pit (L)	1.60	m
Initial depth to Water =	1.31	m
Final depth to water =	1.320	m
Elapsed time (mins)=	60.00	
Top of permeable soil		m
Base of permeable soil		m
Base area=	0.96	m ²
*Av. side area of permeable stratum over test period=	3.014	m ²
Total Exposed area =	3.974	m ²

Infiltration rate (f) = Volume of water used/unit exposed area / unit time

f= 4E-05 m/min or 6.71028E-07 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f-value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT01 (2nd cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712569.377
 726990.853
 79.908

Summary of ground conditions

from	to	Description	Ground water
0.00	0.20	Firm brown TOPSOIL with rootlets	No water
0.20	0.70	Stiff light brown sandy slightly gravelly SILT with occasional cobbles	
0.70	2.00	Stiff to firm brown sandy slightly gravelly cobbly SILT	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.200	0.00
1.200	1.00
1.200	2.00
1.210	3.00
1.210	4.00
1.210	5.00
1.210	6.00
1.210	7.00
1.210	8.00
1.210	9.00
1.210	10.00
1.210	12.00
1.210	14.00
1.210	16.00
1.210	18.00
1.210	20.00
1.210	25.00
1.210	30.00
1.210	40.00
1.210	50.00
1.210	60.00

Field Test

Depth of Pit (D) m
 Width of Pit (B) m
 Length of Pit (L) m

Initial depth to Water = m
 Final depth to water = m
 Elapsed time (mins)=

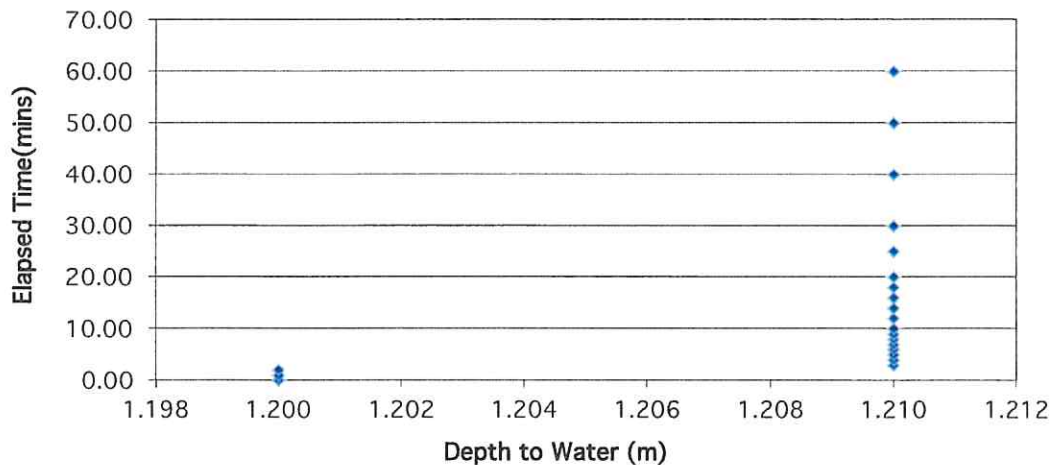
Top of permeable soil m
 Base of permeable soil m

Base area= m²
 *Av. side area of permeable stratum over test period= m²
 Total Exposed area = m²

Infiltration rate (f) = Volume of water used/unit exposed area / unit time

f= 3.6E-05 m/min or 5.98176E-07 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f-value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT02 (1st cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712550.476
 726992.458
 80.112

Summary of ground conditions

from	to	Description	Ground water
0.00	0.20	Firm brown TOPSOIL with rootlets	No water
0.20	0.70	Stiff light brown sandy slightly gravelly SILT with occasional cobbles	
0.70	1.80	Firm to stiff brown sandy slightly gravelly SILT with occasional cobbles	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.500	0.00
1.500	1.00
1.500	2.00
1.500	3.00
1.500	4.00
1.500	5.00
1.500	6.00
1.500	7.00
1.500	8.00
1.500	9.00
1.500	10.00
1.500	12.00
1.500	14.00
1.500	16.00
1.500	18.00
1.500	20.00
1.510	25.00
1.510	30.00
1.510	40.00
1.510	50.00
1.510	60.00

Field Test

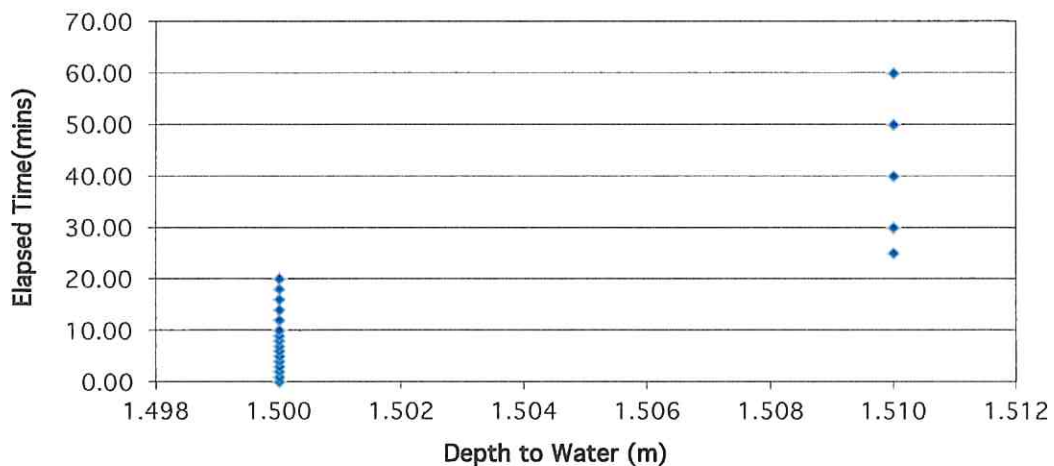
Depth of Pit (D)	1.80	m
Width of Pit (B)	0.60	m
Length of Pit (L)	1.50	m
Initial depth to Water =	1.50	m
Final depth to water =	1.510	m
Elapsed time (mins)=	60.00	
Top of permeable soil		m
Base of permeable soil		m

Base area=	0.9	m ²
*Av. side area of permeable stratum over test period=	2.139	m ²
Total Exposed area =	2.139	m ²

Infiltration rate (f) = Volume of water used/unit exposed area / unit time

f = 7E-05 m/min or 1.16877E-06 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f-value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT02 (2nd cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712550.476
 726992.458
 80.112

Summary of ground conditions

from	to	Description	Ground water
0.00	0.20	Firm brown TOPSOIL with rootlets	No water
0.20	0.70	Stiff light brown sandy slightly gravelly SILT with occasional cobbles	
0.70	1.80	Firm to stiff brown sandy slightly gravelly SILT with occasional cobbles	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.470	0.00
1.470	1.00
1.470	2.00
1.470	3.00
1.470	4.00
1.470	5.00
1.470	6.00
1.470	7.00
1.470	8.00
1.470	9.00
1.470	10.00
1.470	12.00
1.470	14.00
1.470	16.00
1.470	18.00
1.470	20.00
1.470	25.00
1.470	30.00
1.470	40.00
1.480	50.00
1.480	60.00

Field Test

Depth of Pit (D)	1.80	m
Width of Pit (B)	0.60	m
Length of Pit (L)	1.50	m

Initial depth to Water =	1.47	m
Final depth to water =	1.480	m
Elapsed time (mins)=	60.00	

Top of permeable soil		m
Base of permeable soil		m

Base area=	0.9	m ²
*Av. side area of permeable stratum over test period=	1.365	m ²
Total Exposed area =	2.265	m ²

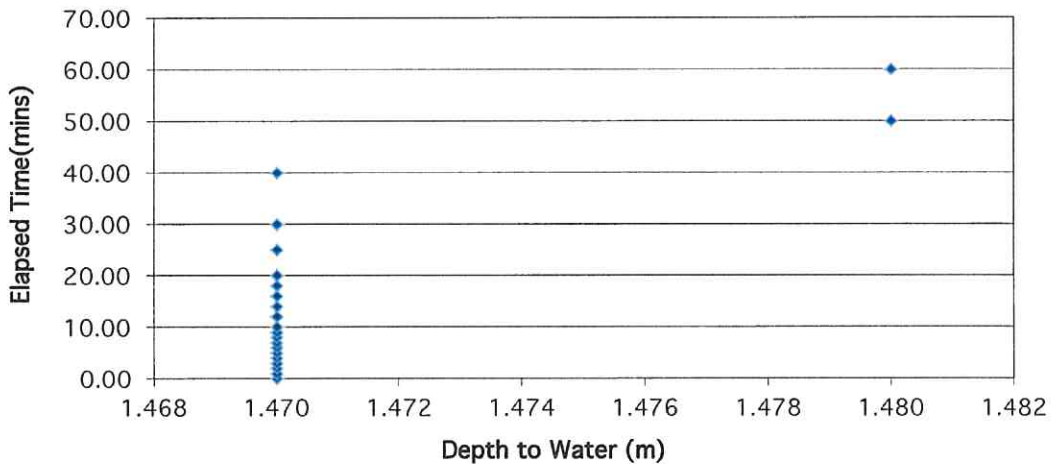
*Av. side area of permeable stratum over test period=

Infiltration rate (f) =

Volume of water used/unit exposed area / unit time

f= 6.6E-05 m/min or 1.10375E-06 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f-value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT03 (1st cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712484.337
 726902.736
 83.542

Summary of ground conditions

from	to	Description	Ground water
0.00	0.15	Firm brown TOPSOIL with rootlets	No water
0.15	0.80	Stiff light brown sandy SILT with occasional gravel and rare cobbles	
0.80	1.75	Stiff brown sandy gravelly cobbly SILT	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.260	0.00
1.260	1.00
1.260	2.00
1.260	3.00
1.260	4.00
1.260	5.00
1.260	6.00
1.260	7.00
1.260	8.00
1.260	9.00
1.260	10.00
1.260	12.00
1.260	14.00
1.260	16.00
1.260	18.00
1.260	20.00
1.260	25.00
1.260	30.00
1.260	40.00
1.260	50.00
1.270	60.00

Field Test

Depth of Pit (D) m
 Width of Pit (B) m
 Length of Pit (L) m

Initial depth to Water = m
 Final depth to water = m
 Elapsed time (mins)=

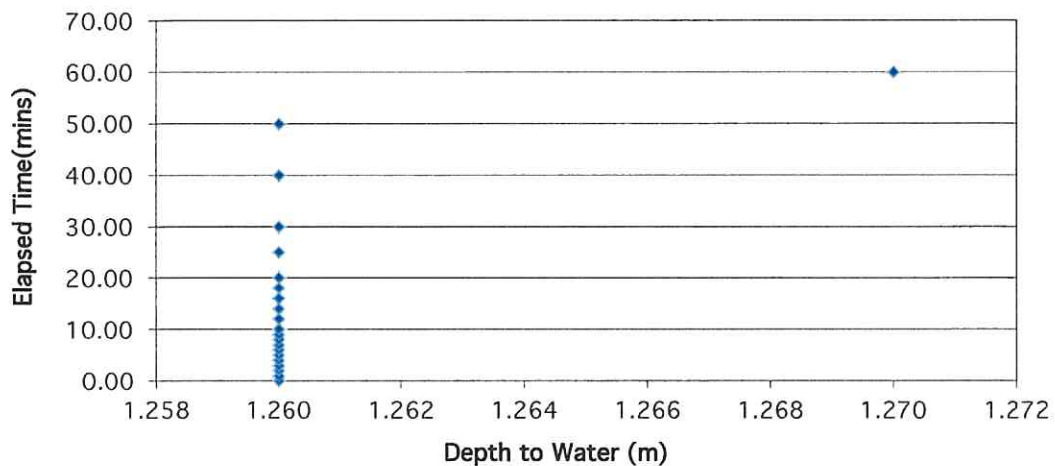
Top of permeable soil m
 Base of permeable soil m

Base area= m²
 *Av. side area of permeable stratum over test period= m²
 Total Exposed area = m²

Infiltration rate (f) = Volume of water used/unit exposed area / unit time

f= 5.1E-05 m/min or 8.51209E-07 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f -value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT03 (2nd cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712484.337
 726902.736
 83.542

Summary of ground conditions

from	to	Description	Ground water
0.00	0.15	Firm brown TOPSOIL with rootlets	No water
0.15	0.80	Stiff light brown sandy SILT with occasional gravel and rare cobbles	
0.80	1.75	Stiff brown sandy gravelly cobbly SILT	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.180	0.00
1.180	1.00
1.180	2.00
1.180	3.00
1.180	4.00
1.180	5.00
1.180	6.00
1.180	7.00
1.180	8.00
1.180	9.00
1.180	10.00
1.180	12.00
1.180	14.00
1.180	16.00
1.180	18.00
1.180	20.00
1.180	25.00
1.180	30.00
1.180	40.00
1.180	50.00
1.180	60.00

Field Test

Depth of Pit (D) m
 Width of Pit (B) m
 Length of Pit (L) m

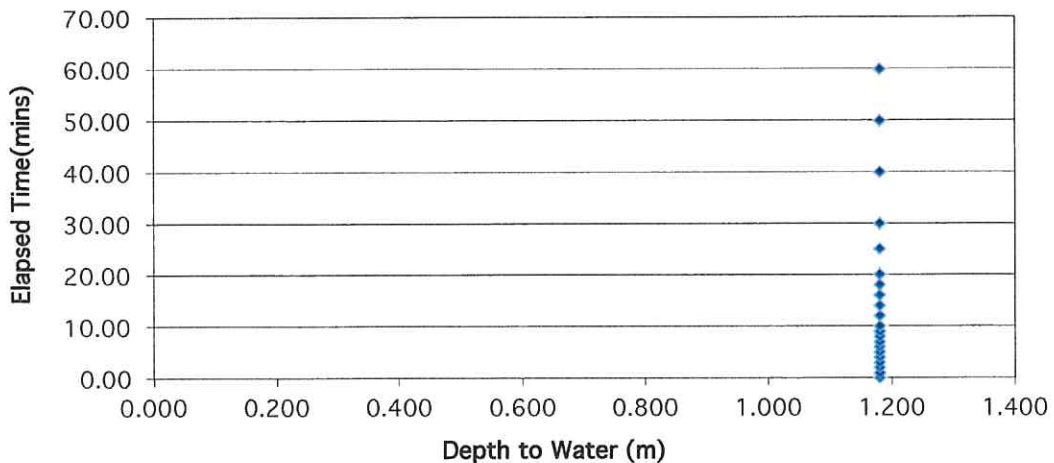
Initial depth to Water = m
 Final depth to water = m
 Elapsed time (mins)=

Top of permeable soil m
 Base of permeable soil m

Base area= m²
 *Av. side area of permeable stratum over test period= m²
 Total Exposed area = m²

Infiltration rate (f) = Volume of water used/unit exposed area / unit time
f= 0 m/min or 0 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f -value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT04 (1st cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712556.069
 726812.607
 83.836

Summary of ground conditions

from	to	Description	Ground water
0.00	0.15	Firm brown TOPSOIL with rootlets	No water
0.15	0.80	Stiff light brown sandy slightly gravelly SILT with occasional cobbles	
0.80	1.75	Firm to stiff brown sandy gravelly cobbly SILT with rare boulders	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.180	0.00
1.180	1.00
1.190	2.00
1.190	3.00
1.190	4.00
1.190	5.00
1.190	6.00
1.190	7.00
1.190	8.00
1.190	9.00
1.190	10.00
1.190	12.00
1.190	14.00
1.190	16.00
1.190	18.00
1.190	20.00
1.190	25.00
1.190	30.00
1.200	40.00
1.200	50.00
1.200	60.00

Field Test

Depth of Pit (D) m
 Width of Pit (B) m
 Length of Pit (L) m

Initial depth to Water = m
 Final depth to water = m
 Elapsed time (mins)=

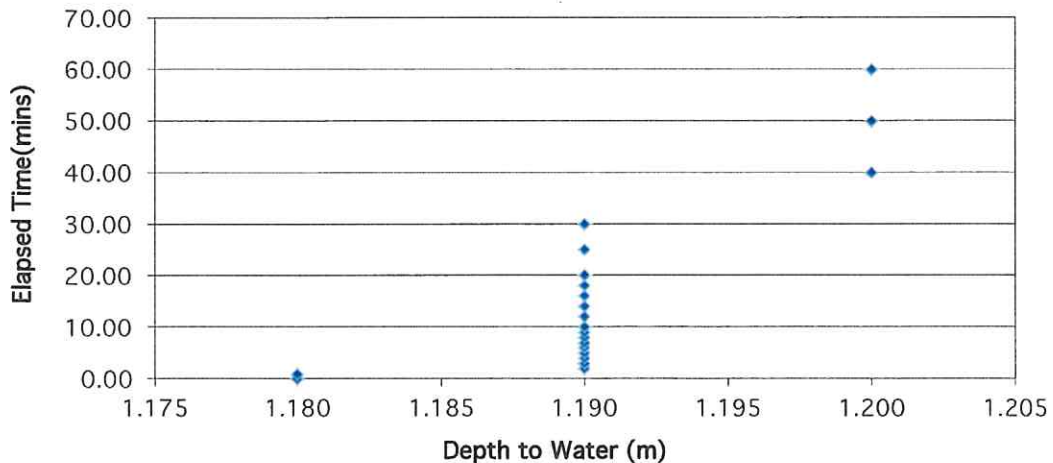
Top of permeable soil m
 Base of permeable soil m

Base area= m²
 *Av. side area of permeable stratum over test period= m²
 Total Exposed area = m²

Infiltration rate (f) = Volume of water used/unit exposed area / unit time

f= 9.2E-05 m/min or 1.53752E-06 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f-value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT04 (2nd cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712556.069
 726812.607
 83.836

Summary of ground conditions

from	to	Description	Ground water
0.00	0.15	Firm brown TOPSOIL with rootlets	No water
0.15	0.80	Stiff light brown sandy slightly gravelly SILT with occasional cobbles	
0.80	1.75	Firm to stiff brown sandy gravelly cobbly SILT with rare boulders	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.000	0.00
1.000	1.00
1.000	2.00
1.000	3.00
1.000	4.00
1.000	5.00
1.000	6.00
1.000	7.00
1.000	8.00
1.000	9.00
1.000	10.00
1.000	12.00
1.000	14.00
1.000	16.00
1.000	18.00
1.000	20.00
1.000	25.00
1.010	30.00
1.010	40.00
1.010	50.00
1.010	60.00

Field Test

Depth of Pit (D)	1.75	m
Width of Pit (B)	0.60	m
Length of Pit (L)	1.50	m
Initial depth to Water =	1.00	m
Final depth to water =	1.010	m
Elapsed time (mins)=	60.00	
Top of permeable soil		m
Base of permeable soil		m
Base area=	0.9	m ²
*Av. side area of permeable stratum over test period=	3.129	m ²
Total Exposed area =	4.029	m ²

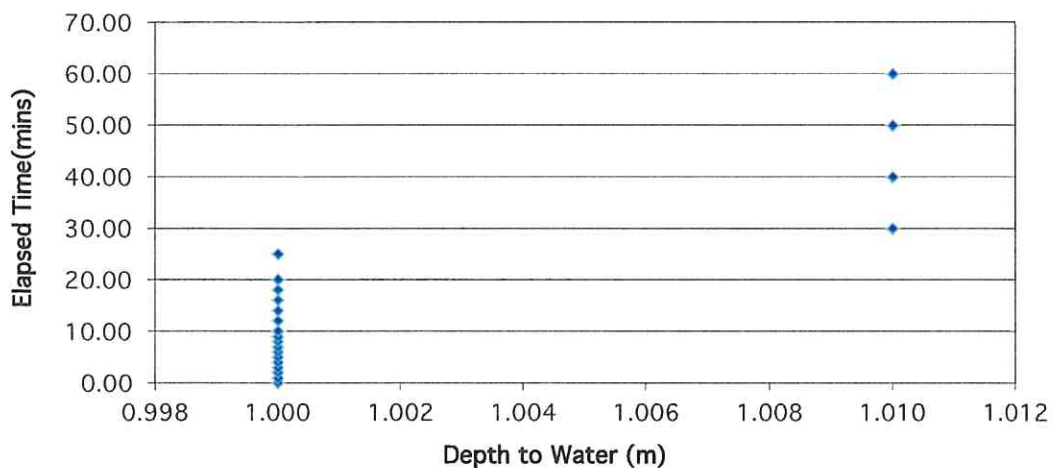
*Av. side area of permeable stratum over test period=

Infiltration rate (f) =

Volume of water used/unit exposed area / unit time

f= 3.7E-05 m/min or 6.20501E-07 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f-value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT05 (1st cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712484.706
 726923.714
 83.146

Summary of ground conditions

from	to	Description	Ground water
0.00	0.15	Firm brown TOPSOIL with roots and rootlets	No water
0.15	0.80	Stiff light brown sandy SILT with occasional gravel	
0.80	1.40	Stiff brown sandy gravelly slightly cobbly SILT	

Field Data

Depth to Water (m)	Elapsed Time (min)
0.980	0.00
0.980	1.00
0.980	2.00
0.980	3.00
0.980	4.00
0.980	5.00
0.980	6.00
0.980	7.00
0.980	8.00
0.980	9.00
0.980	10.00
0.980	12.00
0.980	14.00
0.980	16.00
0.980	18.00
0.980	20.00
0.980	25.00
0.980	30.00
0.980	40.00
0.990	50.00
0.990	60.00

Field Test

Depth of Pit (D) m
 Width of Pit (B) m
 Length of Pit (L) m

Initial depth to Water = m
 Final depth to water = m
 Elapsed time (mins)=

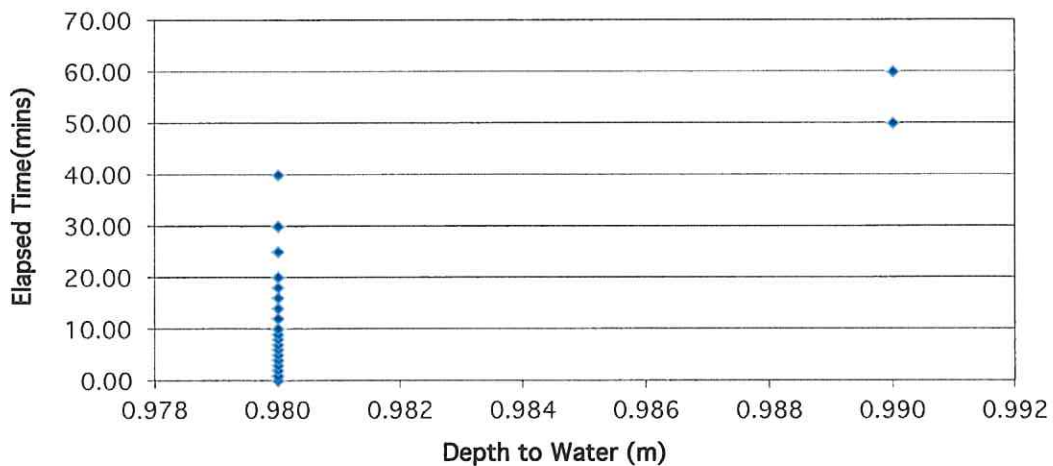
Top of permeable soil m
 Base of permeable soil m

Base area= m²
 *Av. side area of permeable stratum over test period= m²
 Total Exposed area = m²

Infiltration rate (f) = Volume of water used/unit exposed area / unit time

$f = 5.7E-05 \text{ m/min}$ or $9.45895E-07 \text{ m/sec}$

Depth of water vs Elapsed Time (mins)



Soakaway Design f-value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT05 (2nd cycle)
 Engineer DBFL Consulting Engineers
 Date: 09.08.2018

Contract No. 21167
 712484.706
 726923.714
 83.146

Summary of ground conditions

from	to	Description	Ground water
0.00	0.15	Firm brown TOPSOIL with roots and rootlets	No water
0.15	0.80	Stiff light brown sandy SILT with occasional gravel	
0.80	1.40	Stiff brown sandy gravelly slightly cobbly SILT	

Field Data

Depth to Water (m)	Elapsed Time (min)
0.910	0.00
0.910	1.00
0.910	2.00
0.910	3.00
0.910	4.00
0.910	5.00
0.910	6.00
0.910	7.00
0.910	8.00
0.910	9.00
0.910	10.00
0.910	12.00
0.910	14.00
0.910	16.00
0.910	18.00
0.910	20.00
0.910	25.00
0.910	30.00
0.910	40.00
0.910	50.00
0.910	60.00

Field Test

Depth of Pit (D)	1.40	m
Width of Pit (B)	0.60	m
Length of Pit (L)	1.50	m
Initial depth to Water =	0.91	m
Final depth to water =	0.910	m
Elapsed time (mins)=	60.00	
Top of permeable soil		m
Base of permeable soil		m
Base area=	0.9	m ²
*Av. side area of permeable stratum over test period=	2.058	m ²
Total Exposed area =	2.958	m ²

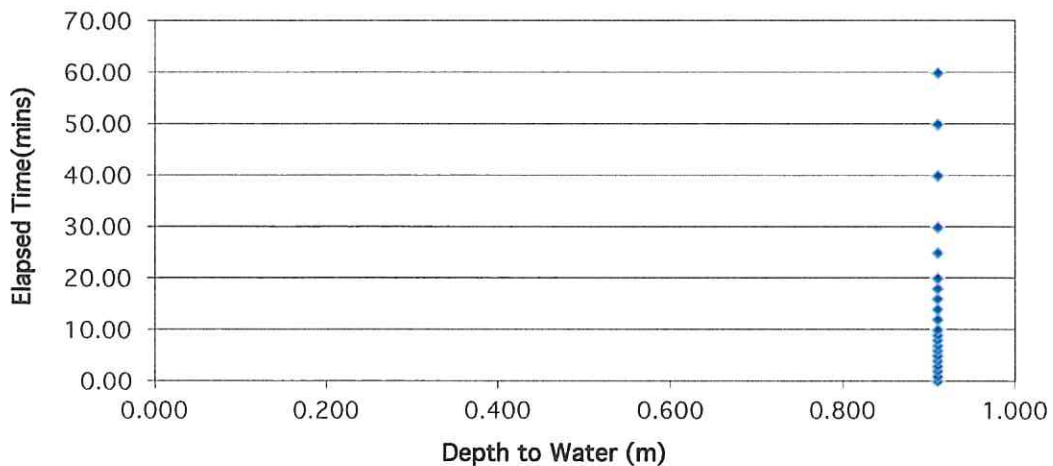
*Av. side area of permeable stratum over test period=

Infiltration rate (f) =

Volume of water used/unit exposed area / unit time

f= 0 m/min or 0 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f -value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT06 (1st cycle)
 Engineer DBFL Consulting Engineers
 Date: 10.08.2018

Contract No. 21167
 712368.625
 726851.292
 86.117

Summary of ground conditions

from	to	Description	Ground water
0.00	0.20	Firm brown TOPSOIL with rootlets	No water
0.20	0.65	Stiff light brown sandy SILT with occasional gravel	
0.80	1.75	Stiff brown/light greyish brown sandy gravelly with occasional cobbles	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.270	0.00
1.270	1.00
1.270	2.00
1.270	3.00
1.270	4.00
1.270	5.00
1.270	6.00
1.270	7.00
1.270	8.00
1.270	9.00
1.270	10.00
1.270	12.00
1.270	14.00
1.270	16.00
1.270	18.00
1.270	20.00
1.270	25.00
1.270	30.00
1.270	40.00
1.280	50.00
1.280	60.00

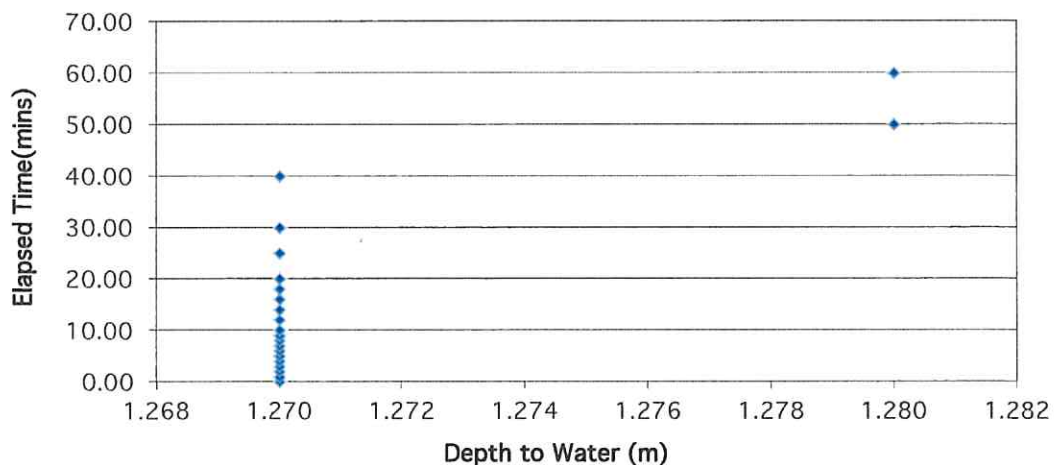
Field Test

Depth of Pit (D)	1.75	m
Width of Pit (B)	0.60	m
Length of Pit (L)	1.50	m
Initial depth to Water =	1.27	m
Final depth to water =	1.280	m
Elapsed time (mins)=	60.00	
Top of permeable soil		m
Base of permeable soil		m
Base area=	0.9	m ²
*Av. side area of permeable stratum over test period=	1.995	m ²
Total Exposed area =	2.895	m ²

Infiltration rate (f) = Volume of water used/unit exposed area / unit time

f= 5.2E-05 m/min or 8.63558E-07 m/sec

Depth of water vs Elapsed Time (mins)



Soakaway Design f-value from field tests

IGSL

Contract: Beech House, Scholarstown
 Test No. IT06 (2nd cycle)
 Engineer DBFL Consulting Engineers
 Date: 10.08.2018

Contract No. 21167
 712368.625
 726851.292
 86.117

Summary of ground conditions

from	to	Description	Ground water
0.00	0.20	Firm brown TOPSOIL with rootlets	No water
0.20	0.65	Stiff light brown sandy SILT with occasional gravel	
0.80	1.75	Stiff brown/light greyish brown sandy gravelly with occasional cobbles	

Field Data

Depth to Water (m)	Elapsed Time (min)
1.200	0.00
1.200	1.00
1.200	2.00
1.200	3.00
1.200	4.00
1.200	5.00
1.200	6.00
1.200	7.00
1.200	8.00
1.200	9.00
1.200	10.00
1.200	12.00
1.200	14.00
1.200	16.00
1.200	18.00
1.200	20.00
1.200	25.00
1.200	30.00
1.200	40.00
1.200	50.00
1.200	60.00

Field Test

Depth of Pit (D) m
 Width of Pit (B) m
 Length of Pit (L) m

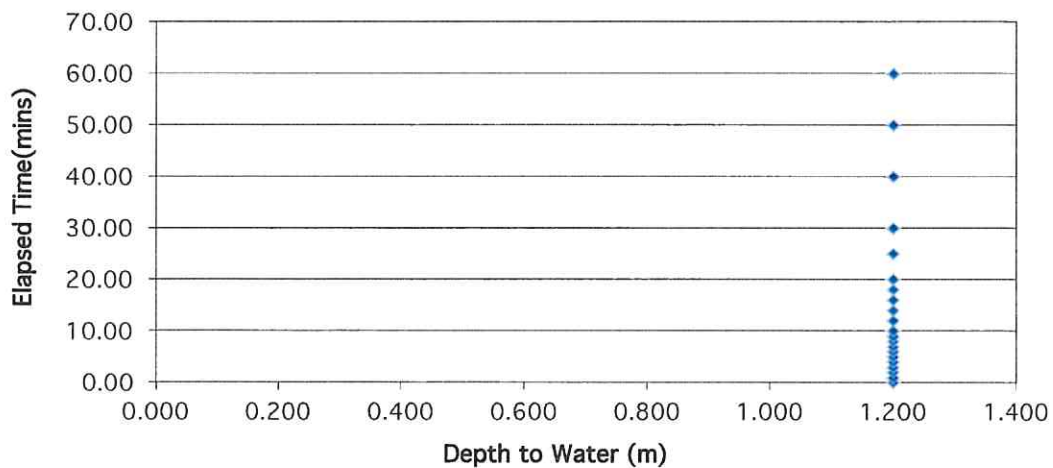
Initial depth to Water = m
 Final depth to water = m
 Elapsed time (mins)=

Top of permeable soil m
 Base of permeable soil m

Base area= m²
 *Av. side area of permeable stratum over test period= m²
 Total Exposed area = m²

Infiltration rate (f) = Volume of water used/unit exposed area / unit time
f= 0 m/min or 0 m/sec

Depth of water vs Elapsed Time (mins)



Appendix VI Laboratory Data

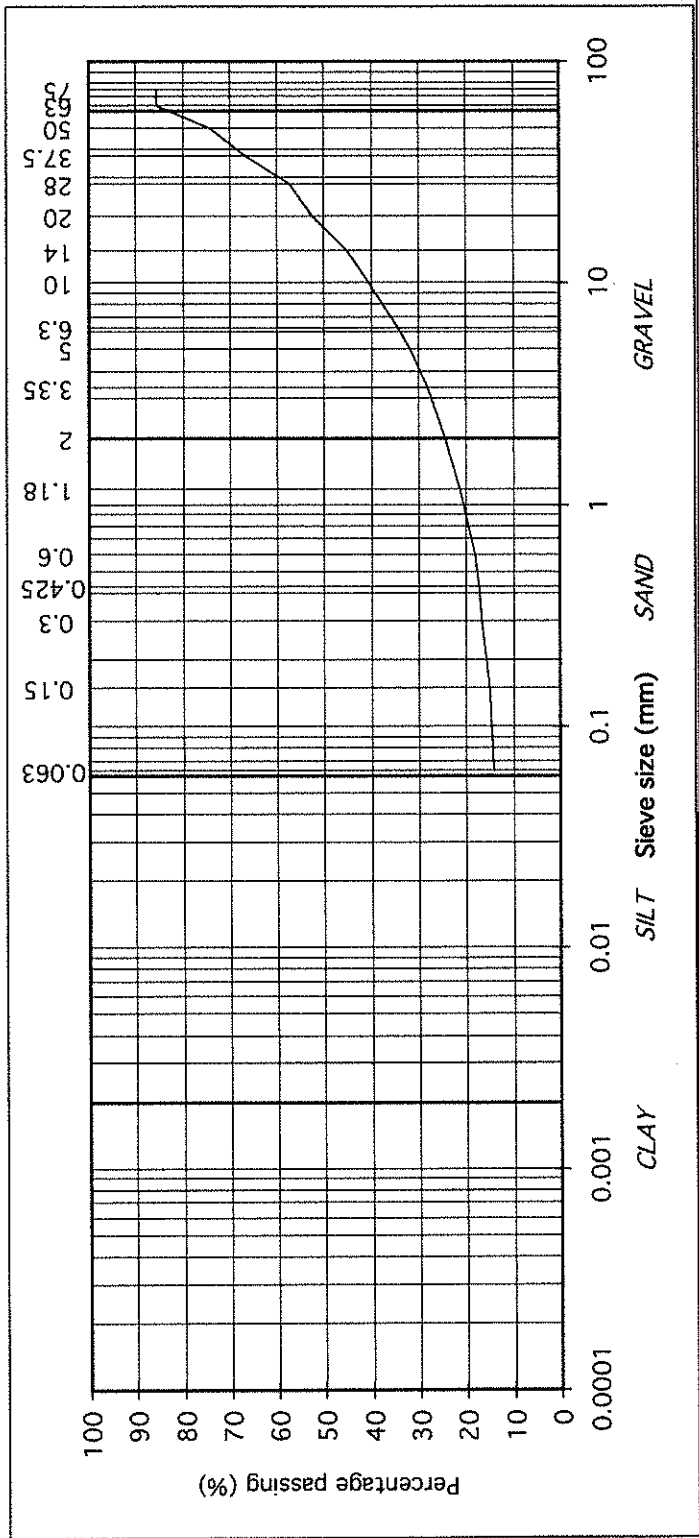
TEST REPORT

Determination of Particle Size Distribution

Tested in accordance with: BS1377:Part2:1990, clause 9.2 & 9.5
(note: Sedimentation stage not accredited)



Contract No:	21167	Report No.	R92782
Contract:	Scholarstown Road, Dublin		
BH/TP:	TP03		
Sample No.	AA94905	Lab. Sample No.	A18/7293
Sample Type:	B		
Depth (m)	2.00	Customer:	DBFL Consulting Engineers, Ormond House, Upper Ormond Quay, Dublin 7, Ireland
Date Received	14/08/2018	Date Testing started	17/08/2018
Description:	Brown silty, sandy, GRAVEL with some cobbles		
Remarks	<p>Note: Clause 9.2 and Clause 9.5 of BS1377:Part 2:1990 have been superseded by 9017892-4:2016</p> <p>Sample size did not meet the requirements of BS1377</p>		



IGSL Ltd Materials Laboratory	Approved by:	Date:	Page no:
	<i>J. Barrett</i>	24/08/18	1 of 1
Persons authorised to approve report: J Barrett (Quality Manager) H Byrne (Laboratory Manager)			

TEST REPORT

Determination of Particle Size Distribution

Tested in accordance with: BS1377:Part2:1990, clause 9.2 & 9.5
(note: Sedimentation stage not accredited)

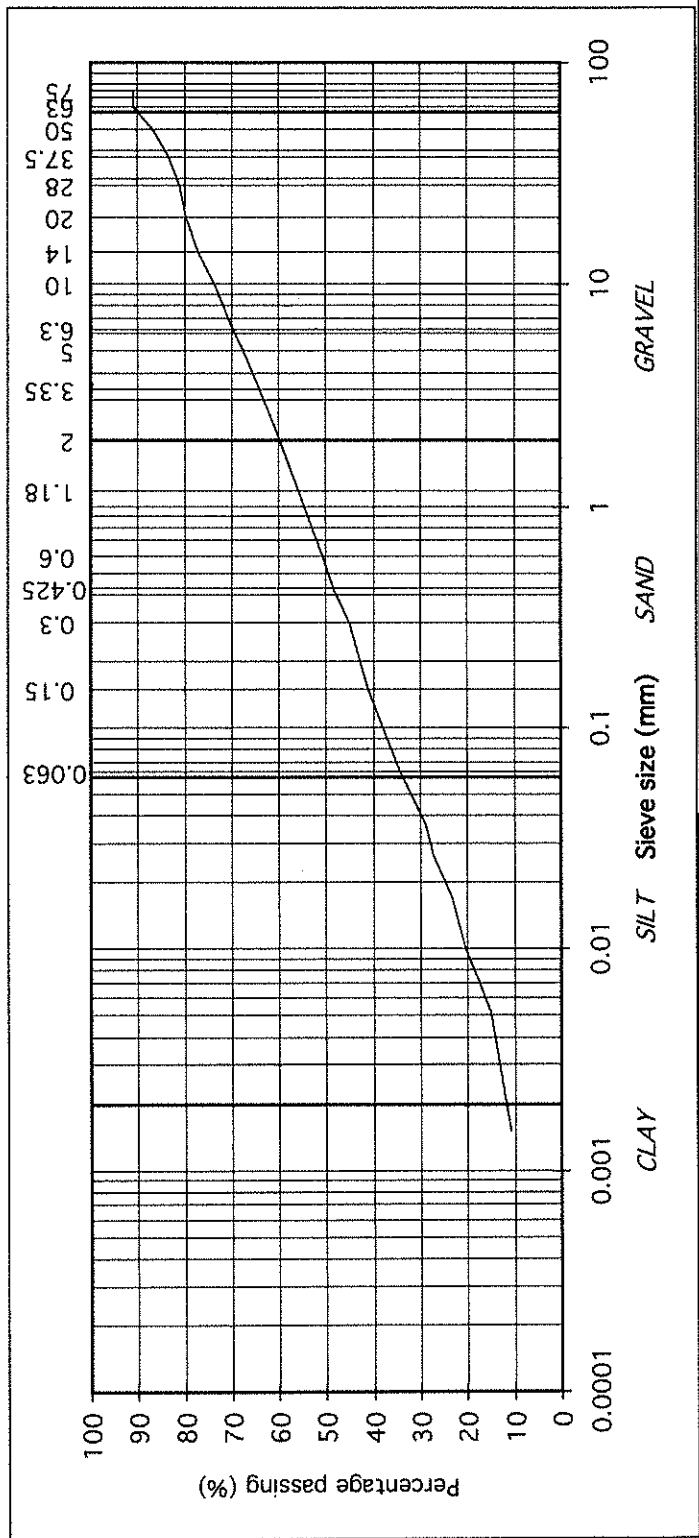


particle size	% passing
75	91
63	91
50	87
37.5	83
28	81
20	80
14	77
10	74
6.3	70
5	67
3.35	64
2	60
1.18	56
0.6	51
0.425	48
0.3	45
0.15	41
0.063	34
0.037	29
0.026	27
0.017	23
0.010	20
0.007	18
0.005	15
0.002	11

COBBLES
GRAVEL
SAND
SILT/CLAY

Contract No: 21167 Report No. R92783
 Contract: Scholarstown Road, Dublin
 BH/TP: TP08
 Sample No. AA81278 Lab. Sample No. A18/7296
 Sample Type: B
 Depth (m) 1.00 Customer: DBFL Consulting Engineers, Ormond House, Upper Ormond Quay, Dublin 7, Ireland
 Date Received 14/08/2018 Date Testing started 18/08/2018
 Description: Brown slightly sandy, slightly gravelly, CLAY with some cobbles

Remarks
Note: Clause 9.2 and Clause 9.5 of BS1377:Part 2:1990 have been superseded by BS17892-4:2016



TEST REPORT

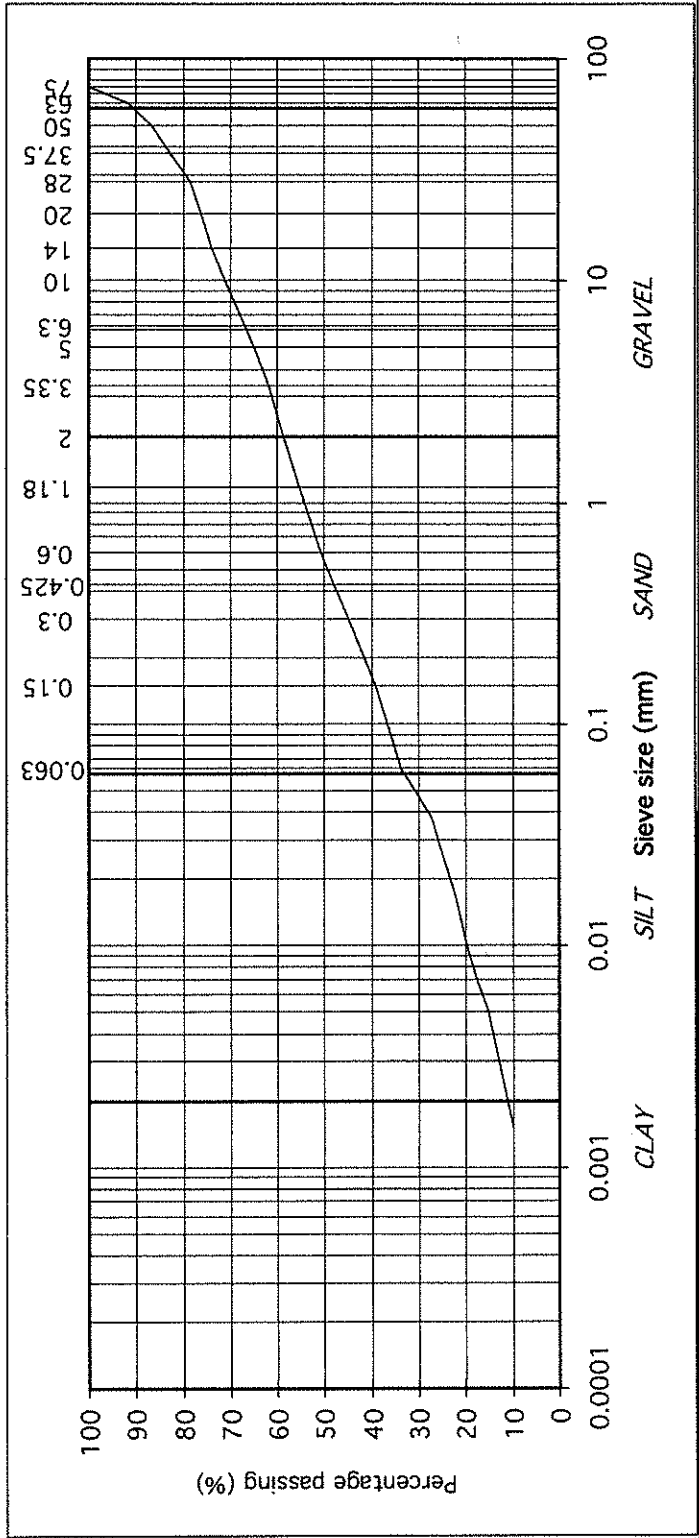
Determination of Particle Size Distribution

Tested in accordance with: BS1377:Part2:1990, clause 9.2 & 9.5
(note: Sedimentation stage not accredited)



particle size	% passing	Contract No: 21167	Report No. R92784
75	100	Contract: Scholarstown Road, Dublin	
63	92	BH/TP: TP12	
50	87	Sample No. AA98927	Lab. Sample No. A18/7298
37.5	83	Sample Type: B	
28	78	Depth (m) 1.00	Customer: DBFL Consulting Engineers, Ormond House, Upper Ormond Quay, Dublin 7, Ireland
20	76	Date Received 14/08/2018	Date Testing started 17/08/2018
14	74	Description: Light brown slightly sandy, gravelly, CLAY with some cobbles	
10	71	Remarks	
6.3	67		
5	65		
3.35	62		
2	59		
1.18	55		
0.6	51		
0.425	48		
0.3	45		
0.15	39		
0.063	34		
0.038	27		
0.027	25		
0.017	22		
0.010	20		
0.007	18		
0.005	15		
0.002	10		

Note: Clause 9.2 and Clause 9.5 of BS1377:Part 2:1990 have been superseded by ISO 17892-4:2016



IGSL Ltd Materials Laboratory		Approved by: <i>[Signature]</i>	Date: 30/08/18	Page no: 1 of 1
Persons authorised to approve report: J Barrett (Quality Manager) H Byrne (Laboratory Manager)				

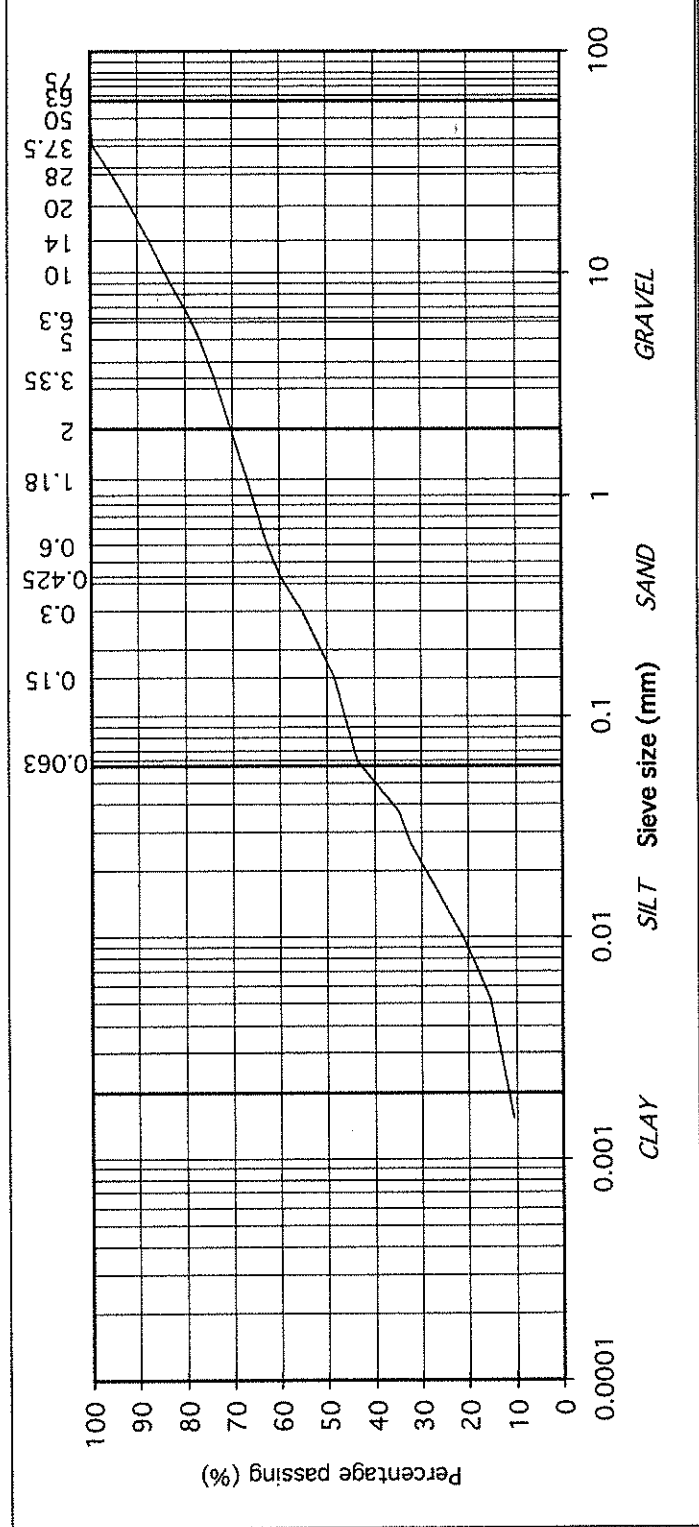
TEST REPORT

Determination of Particle Size Distribution

Tested in accordance with: BS1377:Part2:1990, clause 9.2 & 9.5
(note: Sedimentation stage not accredited)



particle size	% passing	Contract No: 21167	Report No. R92785
75	100	Contract: Scholarstown Road, Dublin	
63	100	BH/TP: TP13	
50	100	Sample No. AA81281	Lab. Sample No. A18/7300
37.5	100	Sample Type: B	
28	95	Depth (m) 2.00	Customer: DBFL Consulting Engineers, Ormond House, Upper Ormond Quay, Dublin 7, Ireland
20	92	Date Received 14/08/2018	Date Testing started 17/08/2018
14	88	Description: Brown slightly sandy, slightly gravelly, CLAY	
10	84	Remarks	
6.3	79		
5	77		
3.35	74		
2	70		
1.18	67		
0.6	63		
0.425	60		
0.3	55		
0.15	48		
0.063	44		
0.037	35		
0.027	32		
0.017	27		
0.010	21		
0.007	18		
0.005	15		
0.002	11		





Final Report

Report No.: 18-25113-1

Initial Date of Issue: 28-Aug-2018

Client: IGSL

Client Address: M7 Business Park
Naas
County Kildare
Ireland

Contact(s): Darren Keogh

Project: 21167 Scholarstown Dublin

Quotation No.: Q17-08989 **Date Received:** 21-Aug-2018

Order No.: **Date Instructed:** 21-Aug-2018

No. of Samples: 8

Turnaround (Wkdays): 5 **Results Due:** 28-Aug-2018

Date Approved: 28-Aug-2018

Approved By:


Details: Glynn Harvey, Laboratory Manager

Client: IGSL	Chemtest Job No.:	18-25113	18-25113	18-25113	18-25113	18-25113	18-25113
Quotation No.: Q17-08989	Chemtest Sample ID.:	674827	674829	674830	674832	674833	674833
Order No.:	Client Sample Ref.:	TP1	TP3	TP5	TP10	TP13	TP13
	Sample Type:	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
	Top Depth (m):	1.00	1.00	1.00	0.50	1.00	1.00
Determinand	Accred.	SOP	Units	LOD			
Ammonium	U	1220	mg/l	0.050	0.19	0.20	0.20
Ammonium	N	1220	mg/kg	0.10	1.9	2.0	2.0
Boron (Dissolved)	U	1450	µg/l	20	< 20	< 20	< 20
Boron (Dissolved)	U	1450	mg/kg	0.20	< 0.20	< 0.20	< 0.20

Client: IGSL	Chemtest Job No.:		18-251113	18-251113	18-251113	18-251113	18-251113	18-251113	18-251113	18-251113	18-251113	18-251113	18-251113
	Quotation No.: Q17-08989	Chemtest Sample ID.:											
Order No.:	Client Sample Ref.:		TP1	TP2	TP3	TP5	TP8	TP10	TP13	TP15	TP10	TP13	TP15
	Sample Type:		SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
	Top Depth (m):		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
	Asbestos Lab:		COVENTRY	COVENTRY	COVENTRY	COVENTRY	COVENTRY	COVENTRY	COVENTRY	COVENTRY	COVENTRY	COVENTRY	COVENTRY
Determinand	Accred.	SOPI	LOD										
ACM Type	U	2192	N/A										
Asbestos Identification	U	2192	%	No Asbestos Detected	No Asbestos Detected	No Asbestos Detected	No Asbestos Detected	No Asbestos Detected	No Asbestos Detected	No Asbestos Detected	No Asbestos Detected	No Asbestos Detected	No Asbestos Detected
Moisture	N	2030	%	11	11	11	11	11	11	11	11	11	11
pH	U	2010	N/A	[A] 8.4	[A] 8.4	[A] 8.4	[A] 8.4	[A] 8.6	[A] 8.6	[A] 8.6	[A] 8.6	[A] 8.4	[A] 8.4
Boron (Hot Water Soluble)	U	2120	mg/kg	< 0.40	< 0.40	< 0.40	< 0.40	< 0.40	< 0.40	< 0.40	< 0.40	< 0.40	< 0.40
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010
Sulphur (Elemental)	U	2180	mg/kg	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Cyanide (Total)	U	2300	mg/kg	[A] < 0.50	[A] < 0.50	[A] < 0.50	[A] < 0.50	[A] < 0.50	[A] < 0.50	[A] < 0.50	[A] < 0.50	[A] < 0.50	[A] < 0.50
Sulphide (Easily Liberatable)	N	2325	mg/kg	[A] 0.82	[A] 4.6	[A] 3.3	[A] 3.3	[A] 3.1	[A] 2.6	[A] 2.6	[A] 3.1	[A] 2.6	[A] 2.6
Sulphate (Acid Soluble)	U	2430	%	[A] 0.019	[A] 0.030	[A] < 0.010	[A] < 0.010	[A] 0.019	[A] 0.020	[A] 0.020	[A] 0.019	[A] 0.020	[A] 0.020
Arsenic	U	2450	mg/kg	44	28	25	25	27	21	21	27	21	21
Barium	U	2450	mg/kg	10	56	46	46	41	40	40	41	40	40
Cadmium	U	2450	mg/kg	0.10	1.5	1.4	1.4	1.6	1.5	1.5	1.6	1.5	1.5
Chromium	U	2450	mg/kg	1.0	19	30	30	27	25	25	27	25	25
Molybdenum	U	2450	mg/kg	2.0	3.3	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0
Antimony	N	2450	mg/kg	2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0
Copper	U	2450	mg/kg	0.50	42	25	25	24	23	23	24	23	23
Mercury	U	2450	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Nickel	U	2450	mg/kg	0.50	72	50	50	47	45	45	47	45	45
Lead	U	2450	mg/kg	0.50	33	24	24	33	24	24	33	24	24
Selenium	U	2450	mg/kg	0.20	1.2	0.81	0.81	< 0.20	0.27	0.27	< 0.20	0.27	0.27
Zinc	U	2450	mg/kg	0.50	150	89	89	90	75	75	90	75	75
Chromium (Trivalent)	N	2490	mg/kg	1.0	45	30	30	27	25	25	27	25	25
Chromium (Hexavalent)	N	2490	mg/kg	0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50
Total Organic Carbon	U	2625	%	[A] 0.24	[A] 0.77	[A] 0.45	[A] 0.45	[A] 0.67	[A] 0.41	[A] 0.41	[A] 0.67	[A] 0.41	[A] 0.41
Mineral Oil	N	2670	mg/kg	10	< 10	< 10	< 10	< 10	< 10	< 10	< 10	< 10	< 10
Aliphatic TPH >C5-C6	N	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aliphatic TPH >C6-C8	N	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aliphatic TPH >C8-C10	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aliphatic TPH >C10-C12	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aliphatic TPH >C12-C16	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aliphatic TPH >C16-C21	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aliphatic TPH >C21-C35	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aliphatic TPH >C35-C44	N	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Total Aliphatic Hydrocarbons	N	2680	mg/kg	5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0
Aromatic TPH >C5-C7	N	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aromatic TPH >C7-C8	N	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aromatic TPH >C8-C10	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aromatic TPH >C10-C12	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0

Client: IGSL	Chemtest Job No.:		Chemtest Sample ID.:		18-251113		18-251113		18-251113		18-251113		18-251113		18-251113	
	Quotation No.: Q17-08989	Chemtest Sample ID.:	Client Sample Ref.:	Sample Type:	Top Depth (m):	Asbestos Lab:	TP1	TP2	TP3	TP5	TP8	TP10	TP13	TP15	SOIL	SOIL
Order No.:	SOIL		COVENTRY		1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.50	1.00	1.00	SOIL	COVENTRY
Determinand	Accred.	SOP	Units	LOD												
Aromatic TPH >C12-C16	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aromatic TPH >C16-C21	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aromatic TPH >C21-C35	U	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Aromatic TPH >C35-C44	N	2680	mg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Total Aromatic Hydrocarbons	N	2680	mg/kg	5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0	[A] < 5.0
Total Petroleum Hydrocarbons	N	2680	mg/kg	10.0	[A] < 10	[A] < 10	[A] < 10	[A] < 10	[A] < 10	[A] < 10	[A] < 10	[A] < 10	[A] < 10	[A] < 10	[A] < 10	[A] < 10
Benzene	U	2760	µg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Toluene	U	2760	µg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Ethylbenzene	U	2760	µg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
m & p-Xylene	U	2760	µg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
o-Xylene	U	2760	µg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Methyl Tert-Butyl Ether	U	2760	µg/kg	1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0	[A] < 1.0
Naphthalene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Acenaphthylene	N	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Acenaphthene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Fluorene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Phenanthrene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Anthracene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Fluoranthene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Pyrene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Benzo[a]anthracene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Chrysene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Benzo[b]fluoranthene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Benzo[k]fluoranthene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Benzo[a]pyrene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Indeno(1,2,3-c,d)Pyrene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Dibenz(a,h)Anthracene	N	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Benzo[g,h,i]perylene	U	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Coronene	N	2800	mg/kg	0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Total Of 17 PAH's	N	2800	mg/kg	2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0
PCB 28	U	2815	mg/kg	0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010
PCB 52	U	2815	mg/kg	0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010
PCB 90+101	U	2815	mg/kg	0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010
PCB 118	U	2815	mg/kg	0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010
PCB 153	U	2815	mg/kg	0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010
PCB 138	U	2815	mg/kg	0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010
PCB 180	U	2815	mg/kg	0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010	[A] < 0.010
Total PCBs (7 Congeners)	N	2815	mg/kg	0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10	[A] < 0.10
Total Phenols	U	2920	mg/kg	0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30

Results - Single Stage WAC

Project: 21167 Schlarstown Dublin

Chemtest Job No: 18-25113

Chemtest Sample ID: 674827

Sample Ref: TP1

Sample ID: 1.00

Top Depth(m):

Bottom Depth(m):

Sampling Date:

Determinand	SOP	Accred.	Units	Landfill Waste Acceptance Criteria		
				Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Total Organic Carbon	2625	U	%	3	5	6
Loss On Ignition	2610	U	%	--	--	10
Total BTEX	2760	U	mg/kg	6	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	1	--	--
TPH Total WAC (Mineral Oil)	2670	U	mg/kg	500	--	--
Total (Of 17) PAH's	2800	N	mg/kg	100	--	--
pH	2010	U		--	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	--	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1450	U	mg/l	0.5	2	25
Barium	1450	U	< 0.0010	20	100	300
Cadmium	1450	U	< 0.00010	0.04	1	5
Chromium	1450	U	< 0.0010	0.5	10	70
Copper	1450	U	< 0.0010	2	50	100
Mercury	1450	U	< 0.00050	0.01	0.2	2
Molybdenum	1450	U	< 0.0010	0.5	10	30
Nickel	1450	U	< 0.0010	0.4	10	40
Lead	1450	U	< 0.0010	0.5	10	50
Antimony	1450	U	< 0.0010	0.06	0.7	5
Selenium	1450	U	< 0.0010	0.1	0.5	7
Zinc	1450	U	< 0.0010	4	50	200
Chloride	1220	U	< 1.0	800	15000	25000
Fluoride	1220	U	0.11	10	150	500
Sulphate	1220	U	< 1.0	1000	20000	50000
Total Dissolved Solids	1020	N	23	4000	60000	100000
Phenol Index	1920	U	< 0.030	1	--	--
Dissolved Organic Carbon	1610	U	6.8	500	800	1000

Solid Information	
Dry mass of test portion/kg	0.090
Moisture (%)	11

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Results - Single Stage WAC

Project: 21167 Schlarstown Dublin

Chemtest Job No: 18-25113

Sample ID: 674829

Sample Ref: TP3

Top Depth(m): 1.00

Bottom Depth(m):

Sampling Date:

Determinand	SOP	Accred.	Units	Landfill Waste Acceptance Criteria		
				Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Total Organic Carbon	2625	U	%	[A] 0.77	5	6
Loss On Ignition	2610	U	%	1.9	--	10
Total BTX	2760	U	mg/kg	[A] < 0.10	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	< 0.10	--	--
TPH Total WAC (Mineral Oil)	2670	U	mg/kg	[A] < 10	--	--
Total (Of 17) PAH's	2800	N	mg/kg	< 2.0	--	--
pH	2010	U		8.3	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	0.034	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	10:1 Eluate	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg	
Arsenic	1450	U	mg/l	< 0.0010	0.5	25
Barium	1450	U		< 0.0010	20	300
Cadmium	1450	U		< 0.00010	0.04	5
Chromium	1450	U		< 0.0010	0.5	70
Copper	1450	U		< 0.0010	2	100
Mercury	1450	U		< 0.00050	0.01	2
Molybdenum	1450	U		< 0.0010	0.5	30
Nickel	1450	U		< 0.0010	0.4	40
Lead	1450	U		< 0.0010	0.5	50
Antimony	1450	U		< 0.0010	0.06	5
Selenium	1450	U		< 0.0010	0.1	7
Zinc	1450	U		< 0.0010	4	200
Chloride	1220	U		< 1.0	800	15000
Fluoride	1220	U		0.13	10	500
Sulphate	1220	U		< 1.0	1000	20000
Total Dissolved Solids	1020	N	26	260	4000	60000
Phenol Index	1920	U	< 0.030	< 0.30	1	--
Dissolved Organic Carbon	1610	U	< 2.5	< 50	500	1000

Solid Information	
Dry mass of test portion/kg	0.090
Moisture (%)	11

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Project: 21167 Schlarstown Dublin

Chemtest Job No: 18-25113

Chemtest Sample ID: 674830

Sample Ref: TP5

Sample ID: 1.00

Top Depth(m):

Bottom Depth(m):

Sampling Date:

Determinand	SOP	Accred.	Units	Landfill Waste Acceptance Criteria		
				Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Total Organic Carbon	2625	U	%	3	5	6
Loss On Ignition	2610	U	%	--	--	10
Total BTEX	2760	U	mg/kg	6	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	1	--	--
TPH Total WAC (Mineral Oil)	2670	U	mg/kg	500	--	--
Total (Of 17) PAH's	2800	N	mg/kg	100	--	--
pH	2010	U		--	>6	--
Acid Neutralisation Capacity	2015	N	mol/kg	--	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1450	U	mg/l	0.5	2	25
Barium	1450	U	< 0.0010	< 0.050	20	300
Cadmium	1450	U	< 0.00010	< 0.010	0.04	5
Chromium	1450	U	< 0.0010	< 0.050	0.5	70
Copper	1450	U	< 0.0010	< 0.050	2	100
Mercury	1450	U	< 0.00050	< 0.0050	0.01	2
Molybdenum	1450	U	< 0.0010	< 0.050	0.5	30
Nickel	1450	U	< 0.0010	< 0.050	0.4	40
Lead	1450	U	< 0.0010	< 0.010	0.5	50
Antimony	1450	U	< 0.0010	< 0.010	0.06	5
Selenium	1450	U	< 0.0010	< 0.010	0.1	7
Zinc	1450	U	< 0.0010	< 0.50	4	200
Chloride	1220	U	< 1.0	< 10	800	15000
Fluoride	1220	U	0.14	1.4	10	500
Sulphate	1220	U	< 1.0	< 10	1000	20000
Total Dissolved Solids	1020	N	34	340	4000	60000
Phenol Index	1920	U	< 0.030	< 0.30	1	100000
Dissolved Organic Carbon	1610	U	4.9	< 50	500	800

Solid Information

Dry mass of test portion/kg	0.090
Moisture (%)	11

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Results - Single Stage WAC

Project: 21167 Scholarstown Dublin

Chemtest Job No: 18-25113

Chemtest Sample ID: 674832

Sample Ref: TP10

Sample ID:
Top Depth(m): 0.50

Bottom Depth(m):
Sampling Date:

Determinand	SOP	Accred.	Units	Landfill Waste Acceptance Criteria		
				Inert Waste Landfill	Stable, Non-reactive hazardous waste in non-hazardous Landfill	Hazardous Waste Landfill
Total Organic Carbon	2625	U	%	[A] 0.67	5	6
Loss On Ignition	2610	U	%	2.0	--	10
Total BTEX	2760	U	mg/kg	[A] < 0.010	--	--
Total PCBs (7 Congeners)	2815	U	mg/kg	< 0.10	--	--
TPH Total WAC (Mineral Oil)	2670	U	mg/kg	[A] < 10	--	--
Total (Of 17) PAH's	2800	N	mg/kg	< 2.0	--	--
pH	2010	U		8.3	> 6	--
Acid Neutralisation Capacity	2015	N	mol/kg	0.037	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate mg/l	10:1 Eluate mg/kg	Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg	
Arsenic	1450	U	< 0.0010	< 0.050	0.5	25
Barium	1450	U	0.0012	< 0.50	20	100
Cadmium	1450	U	< 0.00010	< 0.010	0.04	1
Chromium	1450	U	< 0.0010	< 0.050	0.5	10
Copper	1450	U	< 0.0010	< 0.050	2	50
Mercury	1450	U	< 0.00050	< 0.0050	0.01	0.2
Molybdenum	1450	U	< 0.0010	< 0.050	0.5	10
Nickel	1450	U	< 0.0010	< 0.050	0.4	10
Lead	1450	U	< 0.0010	< 0.010	0.5	10
Antimony	1450	U	< 0.0010	< 0.010	0.06	0.7
Selenium	1450	U	< 0.0010	< 0.010	0.1	0.5
Zinc	1450	U	< 0.0010	< 0.50	4	50
Chloride	1220	U	< 1.0	< 10	800	15000
Fluoride	1220	U	0.17	1.7	10	150
Sulphate	1220	U	< 1.0	< 10	1000	20000
Total Dissolved Solids	1020	N	32	320	4000	60000
Phenol Index	1920	U	< 0.030	< 0.30	1	-
Dissolved Organic Carbon	1610	U	8.7	87	500	800

Solid Information

Dry mass of test portion/kg	0.090
Moisture (%)	10

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Project: 21167, Schlarstown Dublin

Determinand	SOP	Accred.	Units		10:1 Eluate mg/l	10:1 Eluate mg/kg	Landfill Waste Acceptance Criteria		
			%	%			Inert Waste Landfill	Stable, Non- reactive hazardous waste in non- hazardous Landfill	Hazardous Waste Landfill
Total Organic Carbon	2625	U		[A] 0.41			3	5	6
Loss On Ignition	2610	U		1.8			--	--	10
Total BTEX	2760	U		[A] < 0.010			6	--	--
Total PCBs (7 Congeners)	2815	U		< 0.10			1	--	--
TPH Total WAC (Mineral Oil)	2670	U		[A] 17			500	--	--
Total (Of 17) PAH's	2800	N		< 2.0			100	--	--
pH	2010	U		8.4			--	>6	--
Acid Neutralisation Capacity	2015	N		0.041			--	To evaluate	To evaluate
Eluate Analysis							Limit values for compliance leaching test using BS EN 12457 at L/S 10 l/kg		
Arsenic	1450	U		< 0.0010			0.5	2	25
Barium	1450	U		< 0.0010			20	100	300
Cadmium	1450	U		< 0.00010			0.04	1	5
Chromium	1450	U		< 0.0010			0.5	10	70
Copper	1450	U		< 0.0010			2	50	100
Mercury	1450	U		< 0.00050			0.01	0.2	2
Molybdenum	1450	U		< 0.0010			0.5	10	30
Nickel	1450	U		< 0.0010			0.4	10	40
Lead	1450	U		< 0.0010			0.5	10	50
Antimony	1450	U		< 0.0010			0.06	0.7	5
Selenium	1450	U		< 0.0010			0.1	0.5	7
Zinc	1450	U		< 0.0010			4	50	200
Chloride	1220	U		< 1.0			800	15000	25000
Fluoride	1220	U		0.14			10	150	500
Sulphate	1220	U		< 1.0			1000	20000	50000
Total Dissolved Solids	1020	N		32			4000	60000	100000
Phenol Index	1920	U		< 0.030			1	-	-
Dissolved Organic Carbon	1610	U		6.8			500	800	1000

Solid Information	
Dry mass of test portion/kg	0.090
Moisture (%)	10

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Deviations

In accordance with UKAS Policy on Deviating Samples TPS 63. Chemtest have a procedure to ensure 'upon receipt of each sample a competent laboratory shall assess whether the sample is suitable with regard to the requested test(s)'. This policy and the respective holding times applied, can be supplied upon request. The reason a sample is declared as deviating is detailed below. Where applicable the analysis remains UKAS/MCERTs accredited but the results may be compromised.

Sample ID:	Sample Ref:	Sample ID:	Sampled Date:	Deviation Code(s):	Containers Received:
674827	TP1			A	Amber Glass 250ml
674828	TP2			A	Amber Glass 250ml
674829	TP3			A	Amber Glass 250ml
674829	TP3			A	Amber Glass 60ml
674830	TP5			A	Amber Glass 250ml
674830	TP5			A	Amber Glass 60ml
674831	TP8			A	Amber Glass 250ml
674832	TP10			A	Amber Glass 250ml
674832	TP10			A	Amber Glass 60ml
674833	TP13			A	Amber Glass 250ml
674833	TP13			A	Amber Glass 60ml
674834	TP15			A	Amber Glass 250ml

SOP	Title	Parameters included	Method summary
1020	Electrical Conductivity and Total Dissolved Solids (TDS) in Waters	Electrical Conductivity and Total Dissolved Solids (TDS) in Waters	Conductivity Meter
1220	Anions, Alkalinity & Ammonium in Waters	Fluoride; Chloride; Nitrite; Nitrate; Total; Oxidisable Nitrogen (TON); Sulfate; Phosphate; Alkalinity; Ammonium	Automated colorimetric analysis using 'Aquakem 600' Discrete Analyser.
1450	Metals in Waters by ICP-MS	Metals, including: Antimony; Arsenic; Barium; Beryllium; Boron; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Tin; Vanadium; Zinc	Filtration of samples followed by direct determination by inductively coupled plasma mass spectrometry (ICP-MS).
1610	Total/Dissolved Organic Carbon in Waters	Organic Carbon	TOC Analyser using Catalytic Oxidation
1920	Phenols in Waters by HPLC	Phenolic compounds including: Phenol, Cresols, Xylenols, Trimethylphenols Note: Chlorophenols are excluded.	Determination by High Performance Liquid Chromatography (HPLC) using electrochemical detection.
2010	pH Value of Soils	pH	pH Meter
2015	Acid Neutralisation Capacity	Acid Reserve	Titration
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2180	Sulphur (Elemental) in Soils by HPLC	Sulphur	Dichloromethane extraction / HPLC with UV detection
2192	Asbestos	Asbestos	Polarised light microscopy / Gravimetry
2300	Cyanides & Thiocyanate in Soils	Free (or easy liberatable) Cyanide; total Cyanide; complex Cyanide; Thiocyanate	Alkaline extraction followed by colorimetric determination using Automated Flow Injection Analyser.
2325	Sulphide in Soils	Sulphide	Steam distillation with sulphuric acid / analysis by 'Aquakem 600' Discrete Analyser, using N,N-dimethyl-p-phenylenediamine.
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.
2450	Acid Soluble Metals in Soils	Metals, including: Arsenic; Barium; Beryllium; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Vanadium; Zinc	Acid digestion followed by determination of metals in extract by ICP-MS.
2490	Hexavalent Chromium in Soils	Chromium [VI]	Soil extracts are prepared by extracting dried and ground soil samples into boiling water. Chromium [VI] is determined by 'Aquakem 600' Discrete Analyser using 1,5-diphenylcarbazine.
2610	Loss on Ignition	loss on ignition (LOI)	Determination of the proportion by mass that is lost from a soil by ignition at 550°C.
2625	Total Organic Carbon in Soils	Total organic Carbon (TOC)	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2670	Total Petroleum Hydrocarbons (TPH) in Soils by GC-FID	TPH (C6–C40); optional carbon banding, e.g. 3-band – GRO, DRO & LRO*TPH C8–C40	Dichloromethane extraction / GC-FID
2680	TPH A/A Split	Aliphatics: >C5–C6, >C6–C8,>C8–C10, >C10–C12, >C12–C16, >C16–C21, >C21–C35, >C35– C44Aromatics: >C5–C7, >C7–C8, >C8– C10, >C10–C12, >C12–C16, >C16– C21, >C21– C35, >C35– C44	Dichloromethane extraction / GCxGC FID detection

SOP	Title	Parameters included	Method summary
2760	Volatile Organic Compounds (VOCs) in Soils by Headspace GC-MS	Volatile organic compounds, including BTEX and halogenated Aliphatic/Aromatics.(cf. USEPA Method 8260)*please refer to UKAS schedule	Automated headspace gas chromatographic (GC) analysis of a soil sample, as received, with mass spectrometric (MS) detection of volatile organic compounds.
2800	Speciated Polynuclear Aromatic Hydrocarbons (PAH) in Soil by GC-MS	Acenaphthene*; Acenaphthylene; Anthracene*; Benzo[a]Anthracene*; Benzo[a]Pyrene*; Benzo[b]Fluoranthene*; Benzo[ghi]Perylene*; Benzo[k]Fluoranthene; Chrysene*; Dibenz[ah]Anthracene; Fluoranthene*; Fluorene*; Indeno[123cd]Pyrene*; Naphthalene*; Phenanthrene*; Pyrene*	Dichloromethane extraction / GC-MS
2815	Polychlorinated Biphenyls (PCB) ICES7Congeners in Soils by GC-MS	ICES7 PCB congeners	Acetone/Hexane extraction / GC-MS
2920	Phenols in Soils by HPLC	Phenolic compounds including Resorcinol, Phenol, Methylphenols, Dimethylphenols, 1-Naphthol and Trimethylphenols>Note: chlorophenols are excluded.	60:40 methanol/water mixture extraction, followed by HPLC determination using electrochemical detection.
640	Characterisation of Waste (Leaching)	Waste material including soil, sludges and granular waste	ComplianceTest for Leaching of Granular Waste Material and Sludge

Report Information

Key

- U UKAS accredited
- M MCERTS and UKAS accredited
- N Unaccredited
- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
- SN This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
- T This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
- < "less than"
- > "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

Sample Deviation Codes

- A - Date of sampling not supplied
- B - Sample age exceeds stability time (sampling to extraction)
- C - Sample not received in appropriate containers
- D - Broken Container
- E - Insufficient Sample (Applies to LOI in Trommel Fines Only)

Sample Retention and Disposal

All soil samples will be retained for a period of 45 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to:

customerservices@chemtest.co.uk

Appendix VII Site Plan



SITE BOUNDARY IN RED

SITE LAYOUT

DBFL REF:170232

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DATE	11/11/2011	SCALE	AS SHOWN
PROJECT	GENERAL DEVELOPMENT SCHOLARSTOWN		
CLIENT	SCHOLARSTOWN		
DESIGNER	JOHN FLEMING ARCHITECT		
DATE	11/11/2011	SCALE	AS SHOWN
PROJECT	GENERAL DEVELOPMENT SCHOLARSTOWN		
CLIENT	SCHOLARSTOWN		
DESIGNER	JOHN FLEMING ARCHITECT		
DATE	11/11/2011	SCALE	AS SHOWN